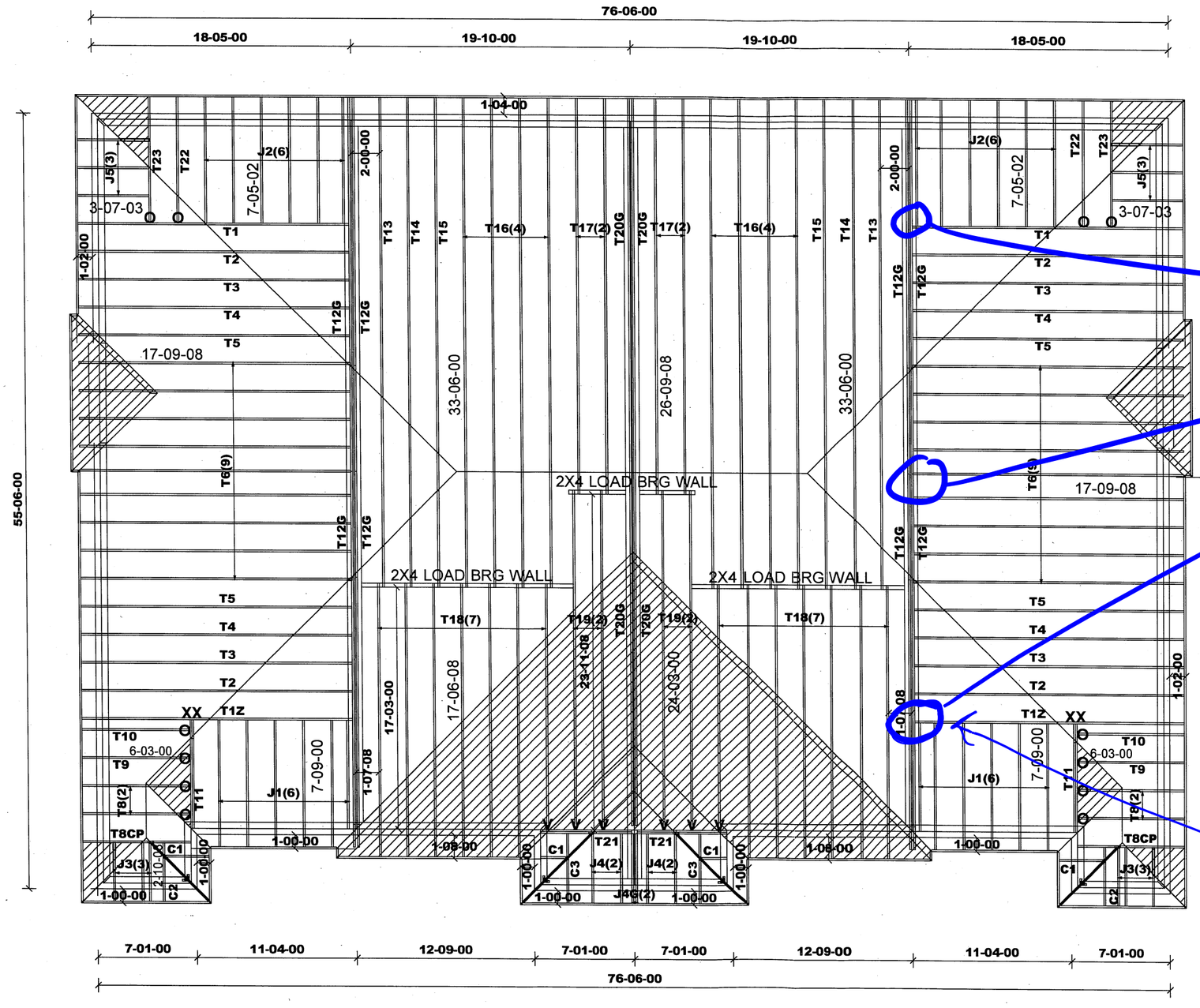
UNIT 1 421918 | UNIT 2 421919 | UNIT 3 421920 | UNIT 4 421921



MIN R20 MUST BE PROVIDED

ALL ROOF PITCHES ARE 4/12
UNLESS OTHERWISE NOTED

ASPHALT SHINGLES
FINISHED OH. VARIES
R.T.M.F.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

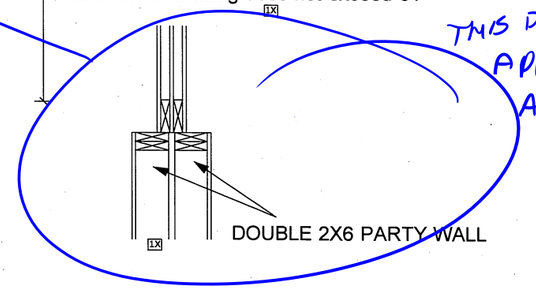
HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26.2 - (XX)

DENOTES CONV. FRAMING

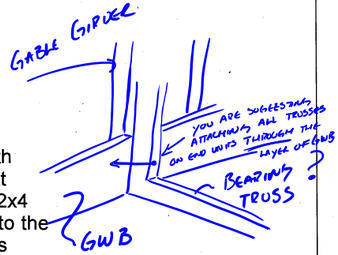
DESIGN CONFORMS WITH OBC 2012 (2019 AMENDMENT)
OCCUPANCY: RESIDENTIAL PART: 9
Ss = 31.3 psf Sr = 8.4 psf

DESIGN LOADS:
TCSL = 25.6 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

All conventional framing to conform with Part 9 of O.B.C. 2012. Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.



THERE MUST BE 2 LAYERS 5/8" G.W.B BETWEEN ALL WOOD IN THESE AREAS NOW! AND THE TWO WALLS CANT TOUCH



THIS DETAIL DOESNT APPLY TO THESE AREAS

M14463

TAMARACK ROOF TRUSSES INC.

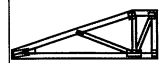
Job Track: 52468	Builder / Location: GOLDCOURT DEVELOPMENTS / HAMILTON	Model / Elevation: UNIT 4/	Mitek ver 8.4.2.286
Plan Log: 205100	Project: 112 SPRINGVALLEY CRESCENT		
Layout ID: 421921	Date: 2021-11-09	Sales: Domenic Ferrelli	Designer: LC

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52468
Builder:	GOLDCOURT DEVELOPMENTS	PlanLog:	205100
Project:	112 SPRINGVALLEY CRESCENT	Layout ID:	421918
Location:	HAMILTON	Ref #	13404
Model:	UNIT 1	Page:	1 of 2
Lot #:		Date:	11-09-2021
Elevation:	UNIT 1	Designer:	
		Sales Rep:	Domenic Ferrelli

Roof Trusses

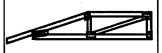
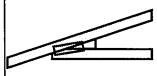
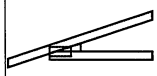
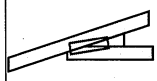
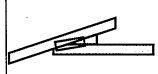
PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	T1 Half Hip Girder	4 /12	17-09-08	3-06-00	2 x 6	1-05-08	6-05 3-00-11	93.12 59.00		
	1	T1Z Half Hip Girder	4 /12	17-09-08	3-06-00	2 x 6	1-05-08	6-05 3-00-11	93.12 59.00		
	2	T2 Half Hip	4 /12	17-09-08	3-11-14	2 x 4	1-05-08	6-05 3-08-11	131.88 83.67		
	2	T3 Half Hip	4 /12	17-09-08	4-07-15	2 x 4	1-05-08	6-05 4-04-11	127.13 77.67		
	2	T4 Half Hip	4 /12	17-09-08	5-03-14	2 x 4	1-05-08	6-05 5-00-11	130.53 81.00		
	2	T5 Half Hip	4 /12	17-09-08	5-11-15	2 x 4	1-05-08	6-05 5-08-11	135.02 86.67		
	9	T6 Monopitch	4 /12	17-09-08	6-08-11	2 x 4	1-05-08	6-05 6-05-08	606.12 370.50		
	2	T8 Common	4 /12	6-03-00	1-09-11	2 x 4	1-05-08	6-05 5-11	38.75 25.00		
	1	T8CP Hip Girder	4 /12	6-03-00	1-08-06	2 x 4	1-05-08	6-05 5-11	19.3 12.50		
	1	T9 Half Hip	4 /12	6-03-00	2-04-12	2 x 4	1-05-08	6-05 2-01-08	21.9 14.67		
	1	T10 Monopitch	4 /12	6-03-00	2-10-08	2 x 4	1-05-08	6-05 2-07-05	22.34 15.33		
	1 2-ply	T11 Half Hip Girder	4 /12	7-09-00	2-07-05	2 x 4		5-11 2-07-05	54.9 38.00		
	2	T12G GABLE	4 /12	25-02-09	6-08-11	2 x 4	1-07-08	7-00 6-05-08	219.56 138.67		
	1	T22 Half Hip	4 /12	7-05-02	2-07-14	2 x 4	1-07-08	7-00 2-04-11	25.79 16.50		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52468
Builder:	GOLDCOURT DEVELOPMENTS	PlanLog:	205100
Project:	112 SPRINGVALLEY CRESCENT	Layout ID:	421918
Location:	HAMILTON	Ref #	13404
Model:	UNIT 1	Page:	2 of 2
Lot #:		Date:	11-09-2021
Elevation:	UNIT 1	Designer:	
		Sales Rep:	Domenic Ferrelli

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	T23 Half Hip Girder	4 /12	**7-05-02**	1-11-14	2 x 4	1-07-08	7-00 1-08-11	25.44 17.00		
	6	J1 Jack-Partial	4 /12	**7-09-00**	3-03-14	2 x 4	1-03-08	5-11 3-00-11	144.97 91.00		
	6	J2 Jack-Partial	4 /12	**7-05-02**	3-03-14	2 x 4	1-07-08	7-00 3-00-11	142.85 91.00		
	3	J3 Jack-Open	4 /12	**2-10-08**	1-08-06	2 x 4	1-03-08	5-11 1-05-03	28.19 18.00		
	3	J5 Jack-Open	4 /12	**3-07-03**	1-11-15	2 x 4	1-05-08	6-05 1-08-11	33.87 22.00		
	1	C1 Jack-Open	4 /12	**1-09-07**	1-04-00	2 x 4	1-03-08 1-01	5-11 1-00-13	6.95 4.67		
	1	C2 Jack-Open	4 /12	**1-09-07**	1-04-00	2 x 4	1-03-08 1-01-01	5-11 1-00-13	8.09 5.33		

TOTAL # TRUSS= **50** TOTAL BFT OF ALL TRUSSES= **1327.18** BFT. TOTAL WEIGHT OF ALL TRSSES **2109.81** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
1	Hardware	HGUS26-2	
6	Hardware	LUS24	

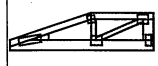
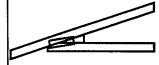
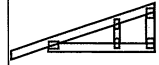
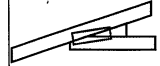
TOTAL NUMBER OF ITEMS= **7**

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52468
Builder:	GOLDCOURT DEVELOPMENTS	PlanLog:	205100
Project:	112 SPRINGVALLEY CRESCENT	Layout ID:	421919
Location:	HAMILTON	Ref #	13404
Model:	UNIT 2	Page:	1 of 2
Lot #:		Date:	11-09-2021
Elevation:	UNIT 2	Designer:	
		Sales Rep:	Domenic Ferrelli

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	T12G GABLE	4 /12	25-02-09	6-08-11	2 x 4	1-07-08	7-00 6-05-08	219.56 138.67		
	1	T13 Hip	4 /12	33-06-00	7-05-08	2 x 4	1-07-08	7-00 6-02-11	146.48 91.17		
	1	T14 Hip	4 /12	33-06-00	8-01-08	2 x 4	1-07-08	7-00 6-02-11	145.54 90.50		
	1	T15 Hip	4 /12	33-06-00	8-09-08	2 x 4	1-07-08	7-00 6-02-11	150.91 93.17		
	4	T16 Common	4 /12	33-06-00	9-03-01	2 x 4	1-07-08	7-00 6-02-11	583.62 362.00		
	2	T17 Common	4 /12	26-09-08	9-03-01	2 x 4	1-07-08	7-00 8-05-08	236.35 146.00		
	7	T18 Monopitch	4 /12	17-06-08	6-09-11	2 x 4	1-11-08	5-11 6-03-14	467.78 284.67		
	2	T19 Monopitch	4 /12	24-03-00	8-06-11	2 x 4		5-11 8-06-11	190.45 117.00		
	2	T20G GABLE	4 /12	25-02-09	9-03-01	2 x 4	1-07-08	7-00 8-11-14	222.6 141.67		
	1 2-ply	T21 Half Hip Girder	4 /12	6-05-08	1-08-06	2 x 4 2 x 6		5-11 1-08-06	49.27 30.67		
	2	J4 Jack-Open	4 /12	3-08-00	1-11-08	2 x 4	1-03-08	5-11 1-08-06	22.5 14.67		
	1	J4G GABLE	4 /12	3-08-00	1-11-08	2 x 4	1-03-08	5-11 1-08-06	12.07 8.50		
	1	C1 Jack-Open	4 /12	1-09-07	1-04-00	2 x 4	1-03-08 1-01	5-11 1-00-13	6.95 4.67		
	1	C3 Jack-Open	4 /12	1-09-07	1-04-00	2 x 4	1-03-08 1-10-09	5-11 1-00-13	8.99 6.00		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52468
Builder:	GOLDCOURT DEVELOPMENTS	PlanLog:	205100
Project:	112 SPRINGVALLEY CRESCENT	Layout ID:	421919
Location:	HAMILTON	Ref #	13404
Model:	UNIT 2	Page:	2 of 2
Lot #:		Date:	11-09-2021
Elevation:	UNIT 2	Designer:	
		Sales Rep:	Domenic Ferrelli

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS

TOTAL # TRUSS= **29** TOTAL BFT OF ALL TRUSSES= **1529.36** BFT. TOTAL WEIGHT OF ALL TRSSES **2463.07** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
3	Hardware	LJS26DS	

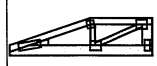
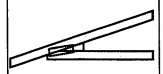
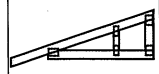
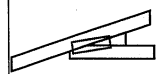
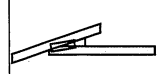
TOTAL NUMBER OF ITEMS= **3**

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52468
Builder:	GOLDCOURT DEVELOPMENTS	PlanLog:	205100
Project:	112 SPRINGVALLEY CRESCENT	Layout ID:	421920
Location:	HAMILTON	Ref #	13404
Model:	UNIT 3	Page:	1 of 2
Lot #:		Date:	11-09-2021
Elevation:	UNIT 3	Designer:	
		Sales Rep:	Domenic Ferrelli

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	2	T12G GABLE	4 /12	25-02-09	6-08-11	2 x 4	1-07-08	7-00 6-05-08	219.56 138.67		
	1	T13 Hip	4 /12	33-06-00	7-05-08	2 x 4	1-07-08	7-00 6-02-11	146.48 91.17		
	1	T14 Hip	4 /12	33-06-00	8-01-08	2 x 4	1-07-08	7-00 6-02-11	145.54 90.50		
	1	T15 Hip	4 /12	33-06-00	8-09-08	2 x 4	1-07-08	7-00 6-02-11	150.91 93.17		
	4	T16 Common	4 /12	33-06-00	9-03-01	2 x 4	1-07-08	7-00 6-02-11	583.62 362.00		
	2	T17 Common	4 /12	26-09-08	9-03-01	2 x 4	1-07-08	7-00 8-05-08	236.35 146.00		
	7	T18 Monopitch	4 /12	17-06-08	6-09-11	2 x 4	1-11-08	5-11 6-03-14	467.78 284.67		
	2	T19 Monopitch	4 /12	24-03-00	8-06-11	2 x 4		5-11 8-06-11	190.45 117.00		
	2	T20G GABLE	4 /12	25-02-09	9-03-01	2 x 4	1-07-08	7-00 8-11-14	222.6 141.67		
	1 2-ply	T21 Half Hip Girder	4 /12	6-05-08	1-08-06	2 x 4 2 x 6		5-11 1-08-06	49.27 30.67		
	2	J4 Jack-Open	4 /12	3-08-00	1-11-08	2 x 4	1-03-08	5-11 1-08-06	22.5 14.67		
	1	J4G GABLE	4 /12	3-08-00	1-11-08	2 x 4	1-03-08	5-11 1-08-06	12.07 8.50		
	1	C1 Jack-Open	4 /12	1-09-07	1-04-00	2 x 4	1-03-08 1-01	5-11 1-00-13	6.95 4.67		
	1	C3 Jack-Open	4 /12	1-09-07	1-04-00	2 x 4	1-03-08 1-10-09	5-11 1-00-13	8.99 6.00		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52468
Builder:	GOLDCOURT DEVELOPMENTS	PlanLog:	205100
Project:	112 SPRINGVALLEY CRESCENT	Layout ID:	421920
Location:	HAMILTON	Ref #	13404
Model:	UNIT 3	Page:	2 of 2
Lot #:		Date:	11-09-2021
Elevation:	UNIT 3	Designer:	
		Sales Rep:	Domenic Ferrelli

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS

TOTAL # TRUSS= **29** TOTAL BFT OF ALL TRUSSES= **1529.36** BFT. TOTAL WEIGHT OF ALL TRSSES **2463.07** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
3	Hardware	LJS26DS	

TOTAL NUMBER OF ITEMS= **3**

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52468
Builder:	GOLDCOURT DEVELOPMENTS	PlanLog:	205100
Project:	112 SPRINGVALLEY CRESCENT	Layout ID:	421921
Location:	HAMILTON	Ref #	13404
Model:	UNIT 4	Page:	1 of 2
Lot #:		Date:	11-09-2021
Elevation:	UNIT 4	Designer:	
		Sales Rep:	Domenic Ferrelli

Roof Trusses

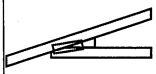
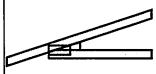
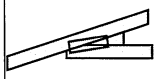
PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	T1 Half Hip Girder	4 /12	**17-09-08**	3-06-00	2 x 6	1-05-08	6-05 3-00-11	93.12 59.00		
	1	T1Z Half Hip Girder	4 /12	**17-09-08**	3-06-00	2 x 6	1-05-08	6-05 3-00-11	93.12 59.00		
	2	T2 Half Hip	4 /12	**17-09-08**	3-11-14	2 x 4	1-05-08	6-05 3-08-11	131.88 83.67		
	2	T3 Half Hip	4 /12	**17-09-08**	4-07-15	2 x 4	1-05-08	6-05 4-04-11	127.13 77.67		
	2	T4 Half Hip	4 /12	**17-09-08**	5-03-14	2 x 4	1-05-08	6-05 5-00-11	130.53 81.00		
	2	T5 Half Hip	4 /12	**17-09-08**	5-11-15	2 x 4	1-05-08	6-05 5-08-11	135.02 86.67		
	9	T6 Monopitch	4 /12	**17-09-08**	6-08-11	2 x 4	1-05-08	6-05 6-05-08	606.12 370.50		
	2	T8 Common	4 /12	**6-03-00**	1-09-11	2 x 4	1-05-08	6-05 5-11	38.75 25.00		
	1	T8CP Hip Girder	4 /12	**6-03-00**	1-08-06	2 x 4	1-05-08	6-05 5-11	19.3 12.50		
	1	T9 Half Hip	4 /12	**6-03-00**	2-04-12	2 x 4	1-05-08	6-05 2-01-08	21.9 14.67		
	1	T10 Monopitch	4 /12	**6-03-00**	2-10-08	2 x 4	1-05-08	6-05 2-07-05	22.34 15.33		
	1 2-ply	T11 Half Hip Girder	4 /12	**7-09-00**	2-07-05	2 x 4		5-11 2-07-05	54.9 38.00		
	2	T12G GABLE	4 /12	**25-02-09**	6-08-11	2 x 4	1-07-08	7-00 6-05-08	219.56 138.67		
	1	T22 Half Hip	4 /12	**7-05-02**	2-07-14	2 x 4	1-07-08	7-00 2-04-11	25.79 16.50		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52468
Builder:	GOLDCOURT DEVELOPMENTS	PlanLog:	205100
Project:	112 SPRINGVALLEY CRESCENT	Layout ID:	421921
Location:	HAMILTON	Ref #	13404
Model:	UNIT 4	Page:	2 of 2
Lot #:		Date:	11-09-2021
Elevation:	UNIT 4	Designer:	
		Sales Rep:	Domenic Ferrelli

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	T23 Half Hip Girder	4 /12	**7-05-02**	1-11-14	2 x 4	1-07-08	7-00 1-08-11	25.44 17.00		
	6	J1 Jack-Partial	4 /12	**7-09-00**	3-03-14	2 x 4	1-03-08	5-11 3-00-11	144.97 91.00		
	6	J2 Jack-Partial	4 /12	**7-05-02**	3-03-14	2 x 4	1-07-08	7-00 3-00-11	142.85 91.00		
	3	J3 Jack-Open	4 /12	**2-10-08**	1-08-06	2 x 4	1-03-08	5-11 1-05-03	28.19 18.00		
	3	J5 Jack-Open	4 /12	**3-07-03**	1-11-15	2 x 4	1-05-08	6-05 1-08-11	33.87 22.00		
	1	C1 Jack-Open	4 /12	**1-09-07**	1-04-00	2 x 4	1-03-08 1-01	5-11 1-00-13	6.95 4.67		
	1	C2 Jack-Open	4 /12	**1-09-07**	1-04-00	2 x 4	1-03-08 1-01-01	5-11 1-00-13	8.09 5.33		

TOTAL # TRUSS= **50** TOTAL BFT OF ALL TRUSSES= **1327.18** BFT. TOTAL WEIGHT OF ALL TRSSES **2109.81** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
1	Hardware	HGUS26-2	
6	Hardware	LUS24	

TOTAL NUMBER OF ITEMS= **7**

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420804	T1	2	1	GOLDCOURT	
				TRUSS DESC. DEVELOPMENTS	

Tamarack Roof Truss, Burlington

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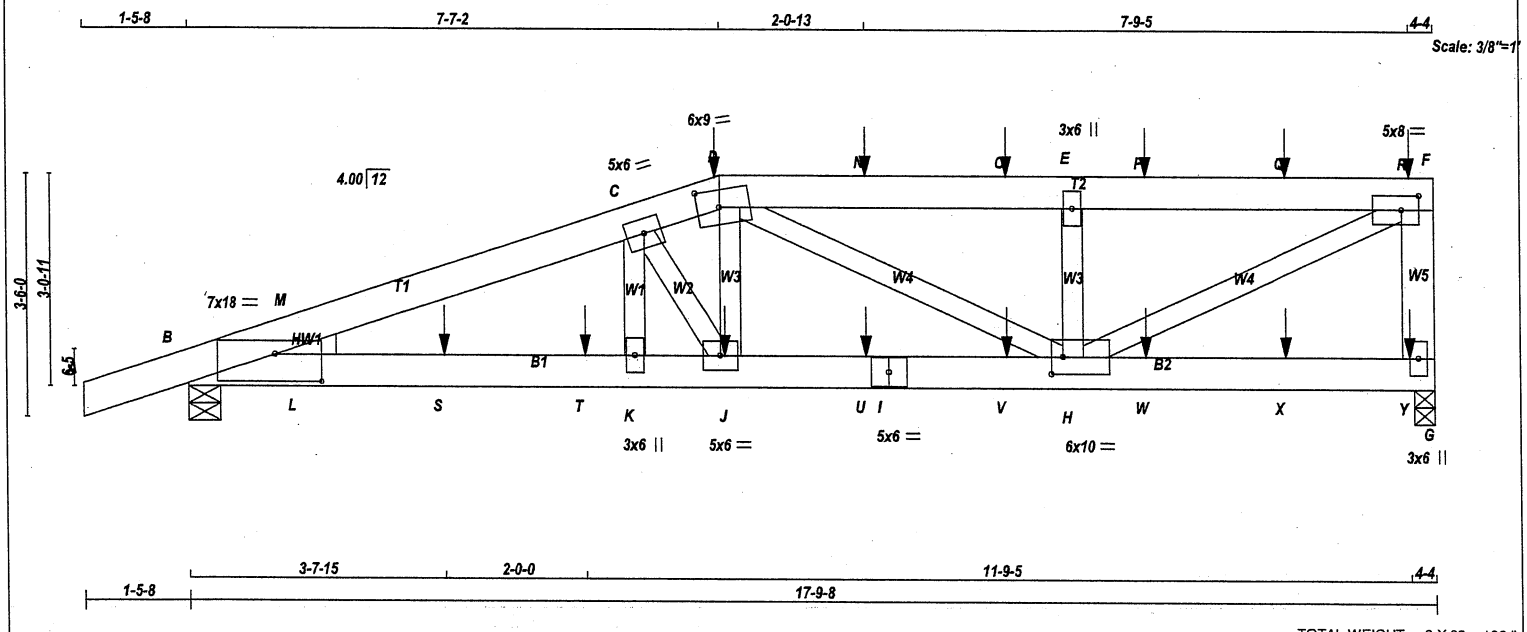

TOTAL WEIGHT = 2 X 93 = 186 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
G - F	2x6	DRY	No.2	SPF
B - I	2x6	DRY	2100F 1.8E	SPF
I - G	2x6	DRY	2100F 1.8E	SPF
ALL WEBS	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-I	MT20	7.0	18.0	4.75	8.00
C	TMWW-t	MT20	5.0	6.0		
D	TTWW-m	MT20	6.0	9.0	3.00	3.75
E	TMW+w	MT20	3.0	6.0		
F	TMVW-t	MT20	5.0	8.0	2.50	3.00
G	BMV1+p	MT20	3.0	6.0		
H	BMWWW-t	MT20	6.0	10.0	3.00	2.00
I	BS-t	MT20	5.0	6.0		
J	BMWW-t	MT20	5.0	6.0		
K	BMW+w	MT20	3.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2136497 1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
G	2553	0	2553	0	0	3-8	3-8	
B	2348	0	2348	0	0	5-8	5-8	2x4 L

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1803	1197 / 0	0 / 0	0 / 0	0 / 0	606 / 0	0 / 0
B	1655	1118 / 0	0 / 0	0 / 0	0 / 0	537 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, B

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.44 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 20	-91.8	-91.8	0.08 (1)	10.00	K- C	0 / 967	0.17 (1)
B- M	-5250 / 0	-91.8	-91.8	0.34 (1)	3.48	C- J	-1413 / 0	0.18 (1)
M- C	-5106 / 0	-91.8	-91.8	0.45 (1)	3.44	J- D	0 / 1467	0.26 (1)
C- D	-4381 / 0	-91.8	-91.8	0.18 (1)	3.98	H- F	0 / 4033	0.71 (1)
D- N	-3605 / 0	-91.8	-91.8	0.27 (1)	4.24	D- H	-672 / 0	0.24 (1)
N- O	-3605 / 0	-91.8	-91.8	0.27 (1)	4.24	H- E	-682 / 0	0.09 (1)
O- E	-3605 / 0	-91.8	-91.8	0.27 (1)	4.24	L- M	0 / 139	0.00 (1)
E- P	-3605 / 0	-91.8	-91.8	0.30 (1)	4.20			
P- Q	-3605 / 0	-91.8	-91.8	0.30 (1)	4.20			
Q- R	-3605 / 0	-91.8	-91.8	0.30 (1)	4.20			
R- F	-3605 / 0	-91.8	-91.8	0.30 (1)	4.20			
G- F	-2112 / 0	0.0	0.0	0.20 (1)	6.96			
B- L	0 / 4869	-18.5	-18.5	0.35 (1)	10.00			
L- S	0 / 4869	-18.5	-18.5	0.56 (1)	10.00			
S- T	0 / 4869	-18.5	-18.5	0.56 (1)	10.00			
T- K	0 / 4869	-18.5	-18.5	0.56 (1)	10.00			
K- J	0 / 4869	-18.5	-18.5	0.41 (1)	10.00			
J- U	0 / 4202	-18.5	-18.5	0.33 (1)	10.00			
U- I	0 / 4202	-18.5	-18.5	0.33 (1)	10.00			
I- V	0 / 4202	-18.5	-18.5	0.33 (1)	10.00			
V- H	0 / 4202	-18.5	-18.5	0.33 (1)	10.00			
H- W	0 / 0	-18.5	-18.5	0.17 (1)	10.00			
W- X	0 / 0	-18.5	-18.5	0.17 (1)	10.00			
X- Y	0 / 0	-18.5	-18.5	0.17 (1)	10.00			
Y- G	0 / 0	-18.5	-18.5	0.17 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	7-7-2	-39	-39	---	BACK	VERT	TOTAL	---	C1
J	7-7-15	-182	-182	---	BACK	VERT	TOTAL	---	C1
N	9-7-15	-33	-33	---	BACK	VERT	TOTAL	---	C1
O	11-7-15	-33	-33	---	BACK	VERT	TOTAL	---	C1
P	13-7-15	-33	-33	---	BACK	VERT	TOTAL	---	C1
Q	15-7-15	-33	-33	---	BACK	VERT	TOTAL	---	C1
R	17-5-4	-54	-54	---	BACK	VERT	TOTAL	---	C1
S	3-7-15	-376	-376	---	BACK	VERT	TOTAL	---	C1
T	5-7-15	-275	-275	---	BACK	VERT	TOTAL	---	C1
U	9-7-15	-182	-182	---	BACK	VERT	TOTAL	---	C1
V	11-7-15	-182	-182	---	BACK	VERT	TOTAL	---	C1
W	13-7-15	-182	-182	---	BACK	VERT	TOTAL	---	C1
X	15-7-15	-182	-182	---	BACK	VERT	TOTAL	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 964 (0.22")

CSI: TC=0.45/1.00 (C-M:1) , BC=0.56/1.00 (K-L:1) , WB=0.71/1.00 (F-H:1) , SSI=0.49/1.00 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (F) (INPUT = 0.90)
JSI METAL= 1.00 (D) (INPUT = 1.00)

CONTINUED ON PAGE 2

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
Y	17-5-4	-187	-187	--	BACK	VERT	TOTAL	--	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2136497

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GOLDCOURT	DRWG NO.
420804	T1Z	2	1	TRUSS DESC.	DEVELOPMENTS	

Tamarack Roof Truss, Burlington

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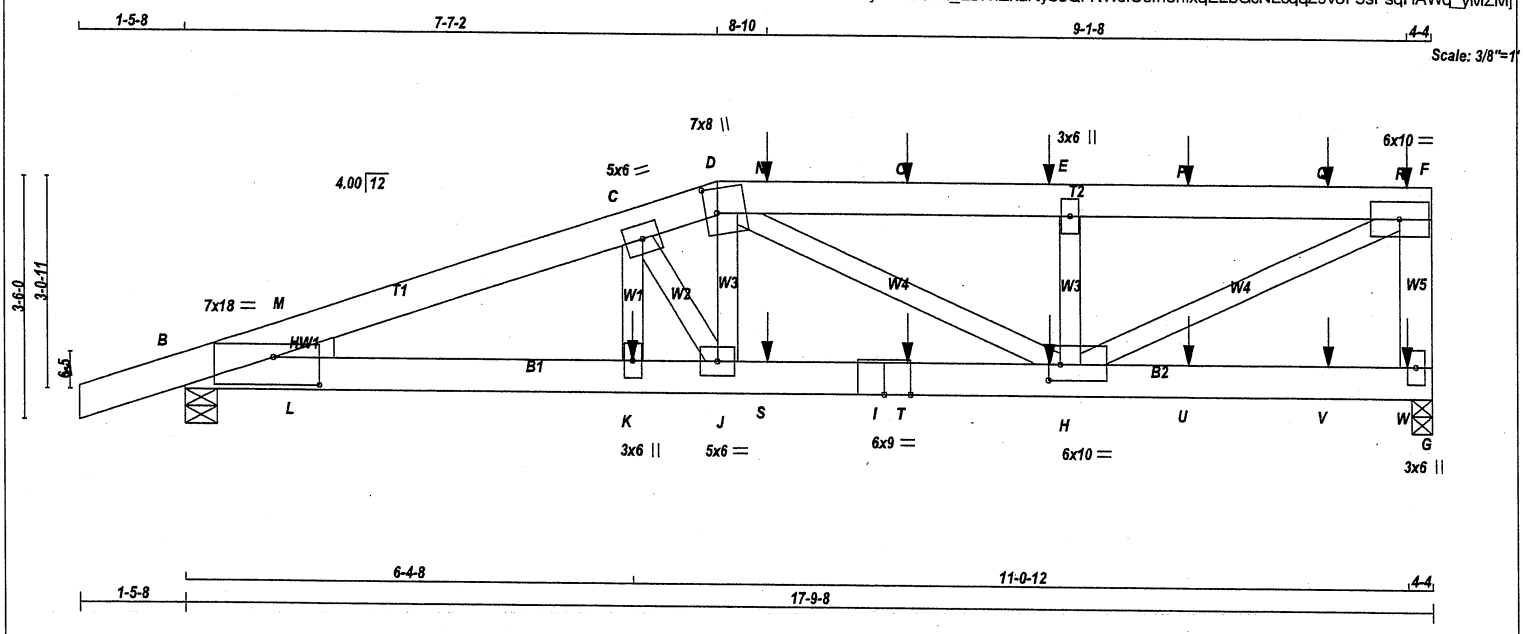

TOTAL WEIGHT = 2 X 93 = 186 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
G - F	2x6	DRY	No.2	SPF
B - I	2x6	DRY	2100F 1.8E	SPF
I - G	2x6	DRY	2100F 1.8E	SPF
ALL WEBS	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-I	MT20	7.0	18.0	4.75	8.00
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	7.0	8.0	4.25	2.00
E	TMW+w	MT20	3.0	6.0		
F	TMVW-t	MT20	6.0	10.0		
G	BMV1+p	MT20	3.0	6.0		
H	BMWWW-t	MT20	6.0	10.0	2.75	2.00
I	BS-t	MT20	6.0	9.0		
J	BMWW-t	MT20	5.0	6.0		
K	BMW+w	MT20	3.0	6.0		

NOTES (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
G	2852	0	2852	0	0	3-8	3-8	
B	2324	0	2324	0	0	5-8	5-8	2x4 L

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	2013	1343 / 0	0 / 0	0 / 0	0 / 0	671 / 0	0 / 0
B	1637	1112 / 0	0 / 0	0 / 0	0 / 0	525 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.36 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 20	-91.8	-91.8	0.08 (1)	10.00
B- M	-5588 / 0	-91.8	-91.8	0.19 (1)	3.56
M- C	-5519 / 0	-91.8	-91.8	0.42 (1)	3.36
C- D	-4900 / 0	-91.8	-91.8	0.15 (1)	3.82
D- N	-3955 / 0	-91.8	-91.8	0.28 (1)	4.06
N- O	-3955 / 0	-91.8	-91.8	0.28 (1)	4.06
O- E	-3955 / 0	-91.8	-91.8	0.28 (1)	4.06
E- P	-3955 / 0	-91.8	-91.8	0.31 (1)	4.03
P- Q	-3955 / 0	-91.8	-91.8	0.31 (1)	4.03
Q- R	-3955 / 0	-91.8	-91.8	0.31 (1)	4.03
R- F	-3955 / 0	-91.8	-91.8	0.31 (1)	4.03
G- F	-2293 / 0	0.0	0.0	0.21 (1)	6.73
B- L	0 / 5236	-18.5	-18.5	0.37 (1)	10.00
L- K	0 / 5236	-18.5	-18.5	0.39 (1)	10.00
K- J	0 / 5236	-18.5	-18.5	0.36 (1)	10.00
J- S	0 / 4744	-18.5	-18.5	0.35 (1)	10.00
S- I	0 / 4744	-18.5	-18.5	0.35 (1)	10.00
I- T	0 / 4744	-18.5	-18.5	0.35 (1)	10.00
T- H	0 / 4744	-18.5	-18.5	0.35 (1)	10.00
H- U	0 / 0	-18.5	-18.5	0.19 (1)	10.00
U- V	0 / 0	-18.5	-18.5	0.19 (1)	10.00
V- W	0 / 0	-18.5	-18.5	0.19 (1)	10.00
W- G	0 / 0	-18.5	-18.5	0.19 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	0 / 862	0.15 (1)
C- J	-1100 / 0	0.14 (1)
J- D	0 / 1635	0.29 (1)
H- F	0 / 4425	0.78 (1)
D- H	-888 / 0	0.31 (1)
H- E	-668 / 0	0.09 (1)
L- M	-86 / 30	0.00 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	12-3-12	-29	-29	---	FRONT	VERT	TOTAL	---	C1
H	12-3-12	-200	-200	---	FRONT	VERT	TOTAL	---	C1
K	6-4-8	-762	-762	---	FRONT	VERT	TOTAL	---	C1
N	8-3-12	-38	-38	---	FRONT	VERT	TOTAL	---	C1
O	10-3-12	-29	-29	---	FRONT	VERT	TOTAL	---	C1
P	14-3-12	-29	-29	---	FRONT	VERT	TOTAL	---	C1
Q	16-3-12	-29	-29	---	FRONT	VERT	TOTAL	---	C1
R	17-5-4	-49	-49	---	FRONT	VERT	TOTAL	---	C1
S	8-3-12	-200	-200	---	FRONT	VERT	TOTAL	---	C1
T	10-3-12	-200	-200	---	FRONT	VERT	TOTAL	---	C1
U	14-3-12	-200	-200	---	FRONT	VERT	TOTAL	---	C1
V	16-3-12	-200	-200	---	FRONT	VERT	TOTAL	---	C1
W	17-5-4	-204	-204	---	FRONT	VERT	TOTAL	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 944 (0.23")

CSI: TC=0.42/1.00 (C-M:1) , BC=0.39/1.00 (K-L:1) , WB=0.78/1.00 (F-H:1) , SSI=0.37/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (H) (INPUT = 0.90)
JSI METAL= 0.83 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2136498

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420804	T2	4	1	GOLDCOURT	
				TRUSS DESC. DEVELOPMENTS	

Tamarack Roof Truss, Burlington

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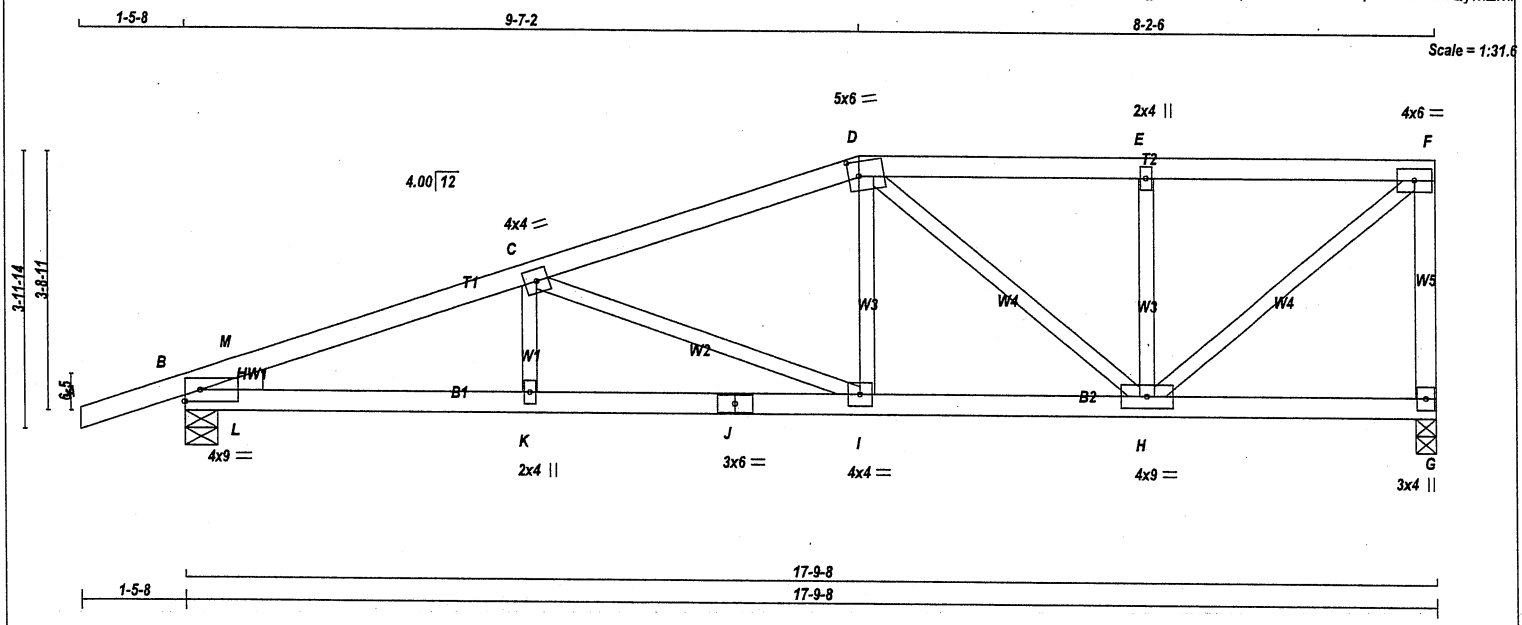

TOTAL WEIGHT = 4 X 66 = 264 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
B - J	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-I	MT20	4.0	9.0		Edge
C	TMWW-t	MT20	4.0	4.0		
D	TTWW-m	MT20	5.0	6.0	2.50	1.75
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BMWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMW+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
G	981	0	981	0	0	3-8	3-8	
B	1119	0	1119	0	0	5-8	5-8	2x4 L

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	694	455 / 0	0 / 0	0 / 0	0 / 0	238 / 0	0 / 0
B	789	532 / 0	0 / 0	0 / 0	0 / 0	256 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.46 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 12	-91.8	-91.8	0.14 (1)	10.00
B- M	-2183 / 0	-91.8	-91.8	0.18 (1)	4.46
M- C	-2036 / 0	-91.8	-91.8	0.20 (1)	4.58
C- D	-1346 / 0	-91.8	-91.8	0.27 (1)	5.30
D- E	-964 / 0	-91.8	-91.8	0.20 (1)	6.10
E- F	-964 / 0	-91.8	-91.8	0.20 (1)	6.10
G- F	-946 / 0	0.0	0.0	0.20 (1)	7.81
B- L	0 / 1933	-18.5	-18.5	0.41 (1)	10.00
L- K	0 / 1933	-18.5	-18.5	0.41 (1)	10.00
K- J	0 / 1933	-18.5	-18.5	0.38 (1)	10.00
J- I	0 / 1933	-18.5	-18.5	0.38 (1)	10.00
I- H	0 / 1278	-18.5	-18.5	0.26 (1)	10.00
H- G	0 / 0	-18.5	-18.5	0.06 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	0 / 83	0.03 (4)
C- I	-714 / 0	0.32 (1)
I- D	0 / 362	0.08 (1)
H- F	0 / 1252	0.28 (1)
D- H	-411 / 0	0.18 (1)
H- E	-462 / 0	0.10 (1)
L- M	0 / 159	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.27/1.00 (C-D:1) , BC=0.41/1.00 (B-L:1) , WB=0.32/1.00 (C-I:1) , SSI=0.18/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (B) (INPUT = 0.90)
JSI METAL= 0.64 (J) (INPUT = 1.00)

Structural component only
DWG# T-2136499

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420804	T3	4	1	GOLDCOURT DEVELOPMENTS	

Tamarack Roof Truss, Burlington

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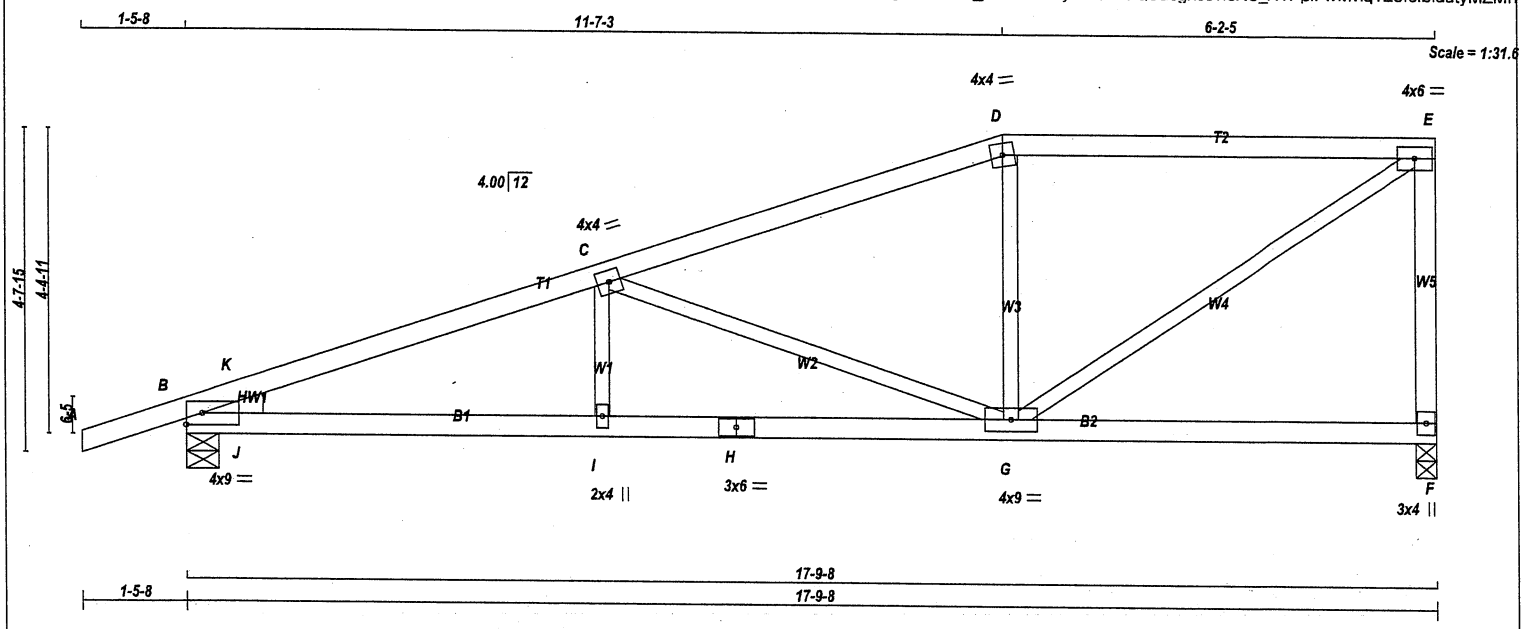

TOTAL WEIGHT = 4 X 64 = 254 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
B - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-l	MT20	4.0	9.0		Edge
C	TMWW-t	MT20	4.0	4.0		
D	TTW-m	MT20	4.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BS-t	MT20	3.0	6.0		
I	BMW+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG	HEEL
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	WEDGE
F	981	0	981	0	0	3-8	3-8	
B	1119	0	1119	0	0	5-8	5-8	2x4 L

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	694	455 / 0	0 / 0	0 / 0	0 / 0	238 / 0	0 / 0
B	789	532 / 0	0 / 0	0 / 0	0 / 0	256 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.47 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 12	-91.8	-91.8	0.14 (1)		10.00	I- C	0 / 105	0.04 (4)
B- K	-2124 / 0	-91.8	-91.8	0.18 (1)		4.51	C- G	-975 / 0	0.67 (1)
K- C	-1979 / 0	-91.8	-91.8	0.34 (1)		4.47	G- D	-173 / 23	0.05 (1)
C- D	-1042 / 0	-91.8	-91.8	0.35 (1)		5.71	G- E	0 / 1169	0.26 (1)
D- E	-969 / 0	-91.8	-91.8	0.47 (1)		5.66	J- K	0 / 114	0.00 (1)
F- E	-936 / 0	0.0	0.0	0.28 (1)		7.81			
B- J	0 / 1885	-18.5	-18.5	0.36 (1)		10.00			
J- I	0 / 1885	-18.5	-18.5	0.36 (1)		10.00			
I- H	0 / 1885	-18.5	-18.5	0.40 (1)		10.00			
H- G	0 / 1885	-18.5	-18.5	0.40 (1)		10.00			
G- F	0 / 0	-18.5	-18.5	0.17 (4)		10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.47/1.00 (D-E:1) , BC=0.40/1.00 (G-I:1) , WB=0.67/1.00 (C-G:1) , SSI=0.23/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (B) (INPUT = 0.90)
JSI METAL= 0.64 (H) (INPUT = 1.00)

Structural component only
DWG# T-2136500

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420804	T4	4	1	GOLDCOURT DEVELOPMENTS	

Tamarack Roof Truss, Burlington

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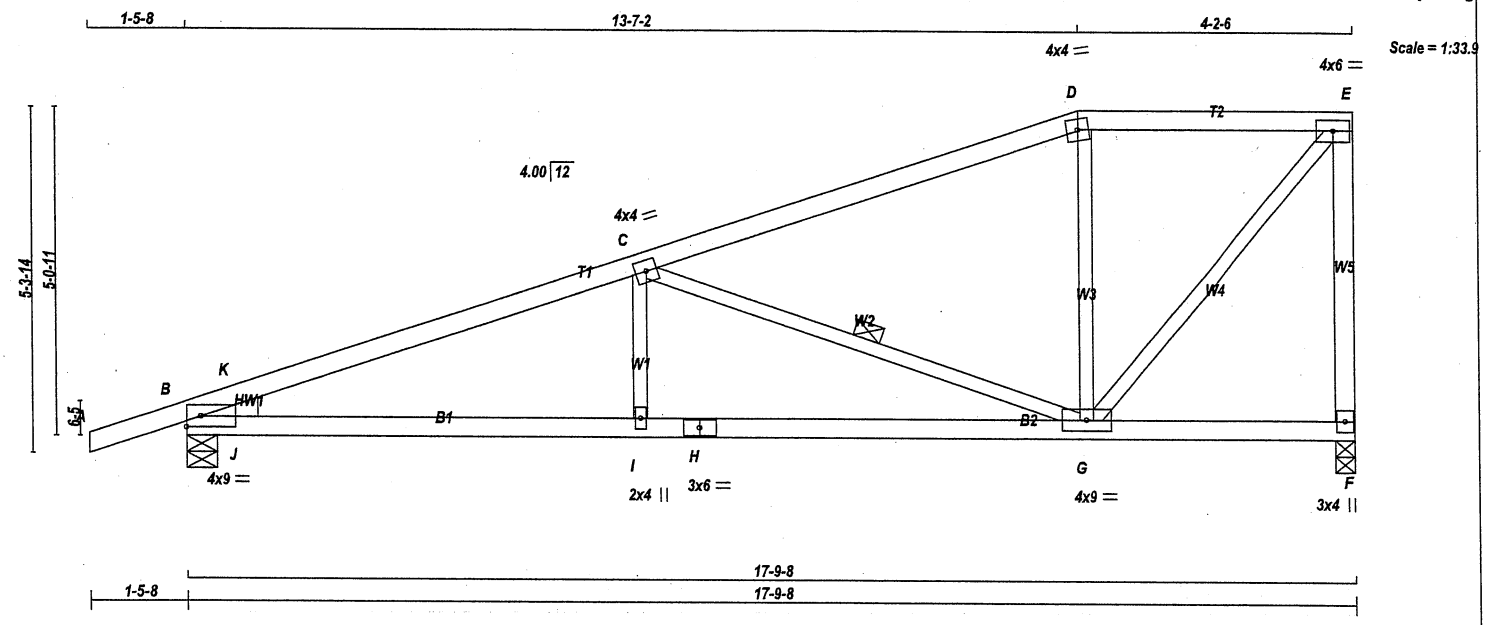

TOTAL WEIGHT = 4 X 65 = 261 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
B - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-I	MT20	4.0	9.0		Edge
C	TMWW-t	MT20	4.0	4.0		
D	TTW-m	MT20	4.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BS-t	MT20	3.0	6.0		
I	BMW+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
F	981	0	981	0	0	3-8	3-8	
B	1119	0	1119	0	0	5-8	5-8	2x4 L

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	694	455 / 0	0 / 0	0 / 0	0 / 0	238 / 0	0 / 0
B	789	532 / 0	0 / 0	0 / 0	0 / 0	256 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.33 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 12	-91.8	-91.8	0.14 (1)	10.00
B- K	-2042 / 0	-91.8	-91.8	0.19 (1)	4.57
K- C	-1906 / 0	-91.8	-91.8	0.50 (1)	4.33
C- D	-718 / 0	-91.8	-91.8	0.49 (1)	6.25
D- E	-658 / 0	-91.8	-91.8	0.21 (1)	6.25
F- E	-958 / 0	0.0	0.0	0.41 (1)	7.81
B- J	0 / 1821	-18.5	-18.5	0.30 (1)	10.00
J- I	0 / 1821	-18.5	-18.5	0.38 (1)	10.00
I- H	0 / 1821	-18.5	-18.5	0.37 (1)	10.00
H- G	0 / 1821	-18.5	-18.5	0.37 (1)	10.00
G- F	0 / 0	-18.5	-18.5	0.14 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- C	0 / 142	0.05 (4)
C- G	-1237 / 0	0.44 (1)
G- D	-218 / 0	0.08 (1)
G- E	0 / 1009	0.23 (1)
J- K	0 / 102	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.50/1.00 (C-K:1) , BC=0.38/1.00 (I-J:1) , WB=0.44/1.00 (C-G:1) , SSI=0.27/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (B) (INPUT = 0.90)
JSI METAL= 0.52 (H) (INPUT = 1.00)

Structural component only
DWG# T-2136501

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GOLDCOURT	DRWG NO.
420804	T5	4	1	TRUSS DESC.	DEVELOPMENTS	

Tamarack Roof Truss, Burlington

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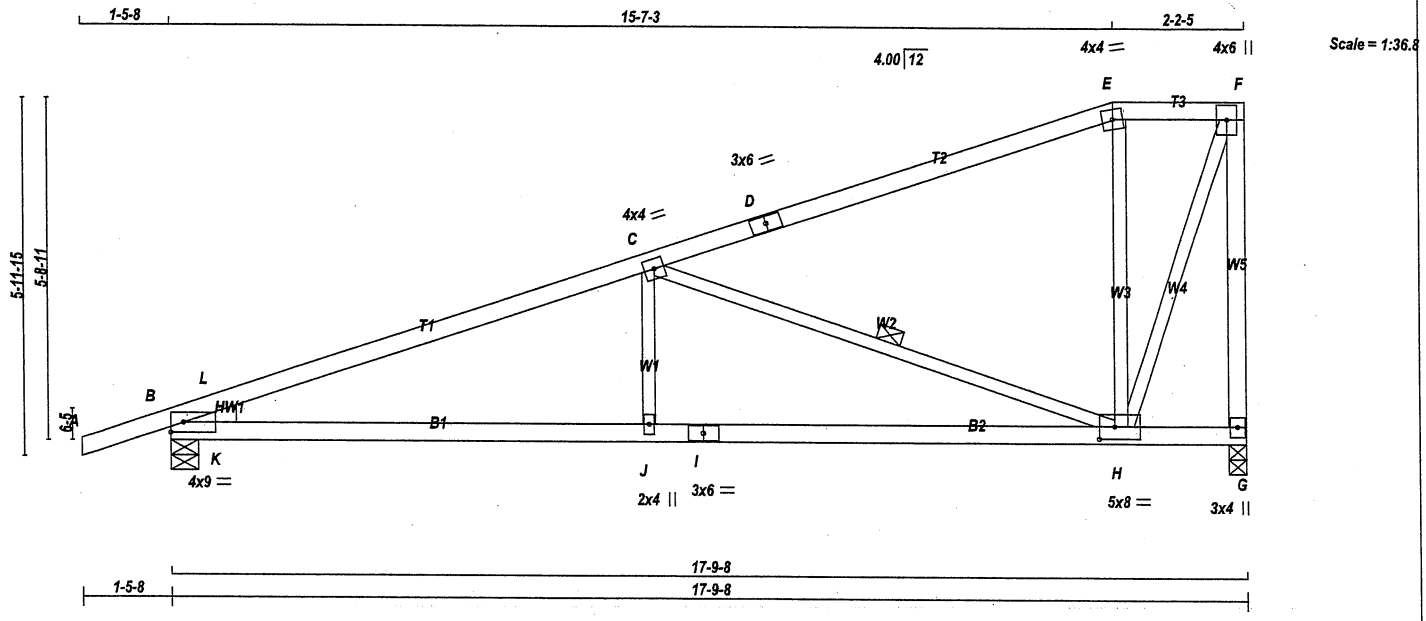

TOTAL WEIGHT = 4 X 68 = 270 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
B - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-I	MT20	4.0	9.0		Edge
C	TMWW-t	MT20	4.0	4.0		
D	TS-t	MT20	3.0	6.0		
E	TTW-m	MT20	4.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	8.0	2.50	3.00
I	BS-t	MT20	3.0	6.0		
J	BMW+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
G	981	0	981	0	0	3-8	3-8	
B	1119	0	1119	0	0	5-8	5-8	2x4 L

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	694	455 / 0	0 / 0	0 / 0	0 / 0	238 / 0	0 / 0
B	789	532 / 0	0 / 0	0 / 0	0 / 0	256 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, B

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.08 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 12	-91.8	-91.8	0.14 (1)	10.00
B- L	-1941 / 0	-91.8	-91.8	0.20 (1)	4.66
L- C	-1817 / 0	-91.8	-91.8	0.69 (1)	4.08
C- D	-390 / 0	-91.8	-91.8	0.66 (1)	6.25
D- E	-390 / 0	-91.8	-91.8	0.66 (1)	6.25
E- F	-343 / 0	-91.8	-91.8	0.06 (1)	6.25
G- F	-992 / 0	0.0	0.0	0.60 (1)	7.81
B- K	0 / 1742	-18.5	-18.5	0.35 (1)	10.00
K- J	0 / 1742	-18.5	-18.5	0.42 (1)	10.00
J- I	0 / 1742	-18.5	-18.5	0.42 (1)	10.00
I- H	0 / 1742	-18.5	-18.5	0.42 (1)	10.00
H- G	0 / 0	-18.5	-18.5	0.17 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- C	0 / 170	0.06 (4)
C- H	-1485 / 0	0.70 (1)
H- E	-263 / 0	0.13 (1)
H- F	0 / 956	0.22 (1)
K- L	-13 / 101	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.69/1.00 (C-L:1) , BC=0.42/1.00 (J-K:1) , WB=0.70/1.00 (C-H:1) , SSI=0.32/1.00 (C-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.60 (I) (INPUT = 1.00)

Structural component only
DWG# T-2136502

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420804	T6	18	1	GOLDCOURT DEVELOPMENTS	

Tamarack Roof Truss, Burlington

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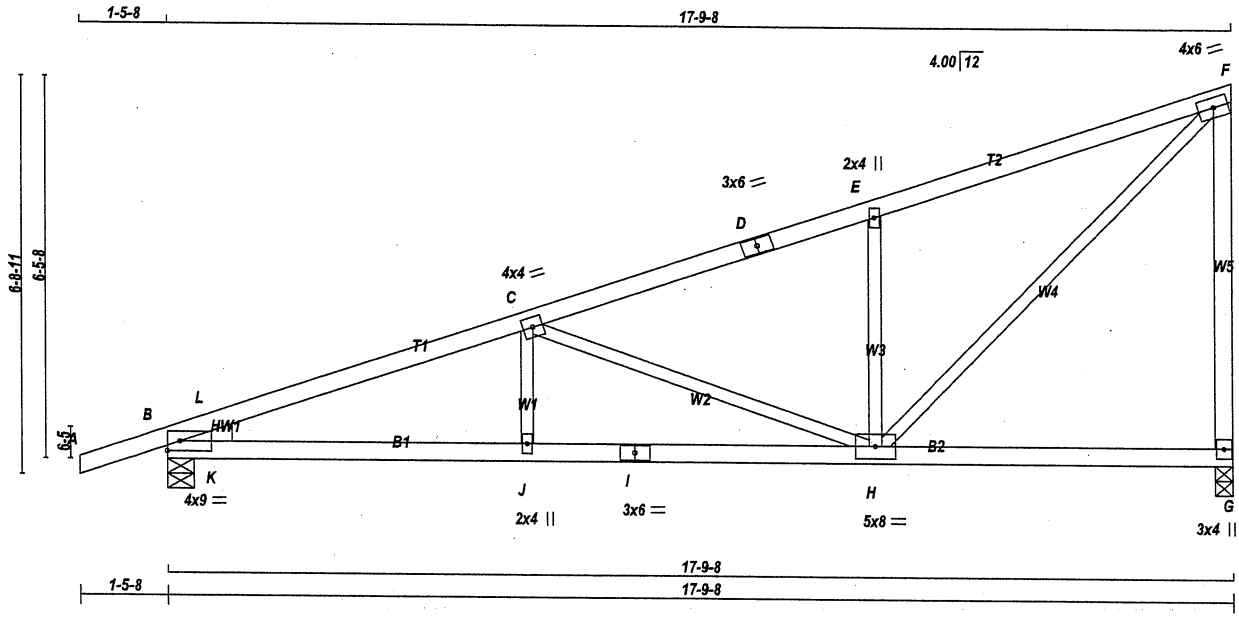

TOTAL WEIGHT = 18 X 67 = 1212 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
B - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-I	MT20	4.0	9.0		Edge
C	TMWW-t	MT20	4.0	4.0		
D	TS-t	MT20	3.0	6.0		
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BS-t	MT20	3.0	6.0		
J	BMW+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
G	981	0	981	0	0	3-8	3-8	
B	1119	0	1119	0	0	5-8	5-8	2x4 L

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	694	455 / 0	0 / 0	0 / 0	0 / 0	238 / 0	0 / 0
B	789	532 / 0	0 / 0	0 / 0	0 / 0	256 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.54 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF) CSI (LC)	MAX UNBRAC LENGTH
FR-TO					
A- B	0 / 12	-91.8	-91.8	0.14 (1)	10.00
B- L	-2067 / 0	-91.8	-91.8	0.20 (1)	4.54
L- C	-1943 / 0	-91.8	-91.8	0.27 (1)	4.61
C- D	-1085 / 0	-91.8	-91.8	0.38 (1)	5.59
D- E	-1085 / 0	-91.8	-91.8	0.38 (1)	5.59
E- F	-1103 / 0	-91.8	-91.8	0.41 (1)	5.51
G- F	-935 / 0	0.0	0.0	0.79 (1)	7.81
B- K	0 / 1846	-18.5	-18.5	0.31 (1)	10.00
K- J	0 / 1846	-18.5	-18.5	0.36 (1)	10.00
J- I	0 / 1846	-18.5	-18.5	0.38 (1)	10.00
I- H	0 / 1846	-18.5	-18.5	0.38 (1)	10.00
H- G	0 / 0	-18.5	-18.5	0.16 (4)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		
J- C	0 / 110	0.04 (4)
C- H	-867 / 0	0.60 (1)
H- E	-620 / 0	0.18 (1)
H- F	0 / 1476	0.33 (1)
K- L	0 / 96	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.79/1.00 (F-G:1) , BC=0.38/1.00 (H-J:1) , WB=0.60/1.00 (C-H:1) , SSI=0.25/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (F) (INPUT = 0.90)
JSI METAL= 0.61 (I) (INPUT = 1.00)

Structural component only
DWG# T-2136503

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420804	T8	4	1	GOLDCOURT DEVELOPMENTS	

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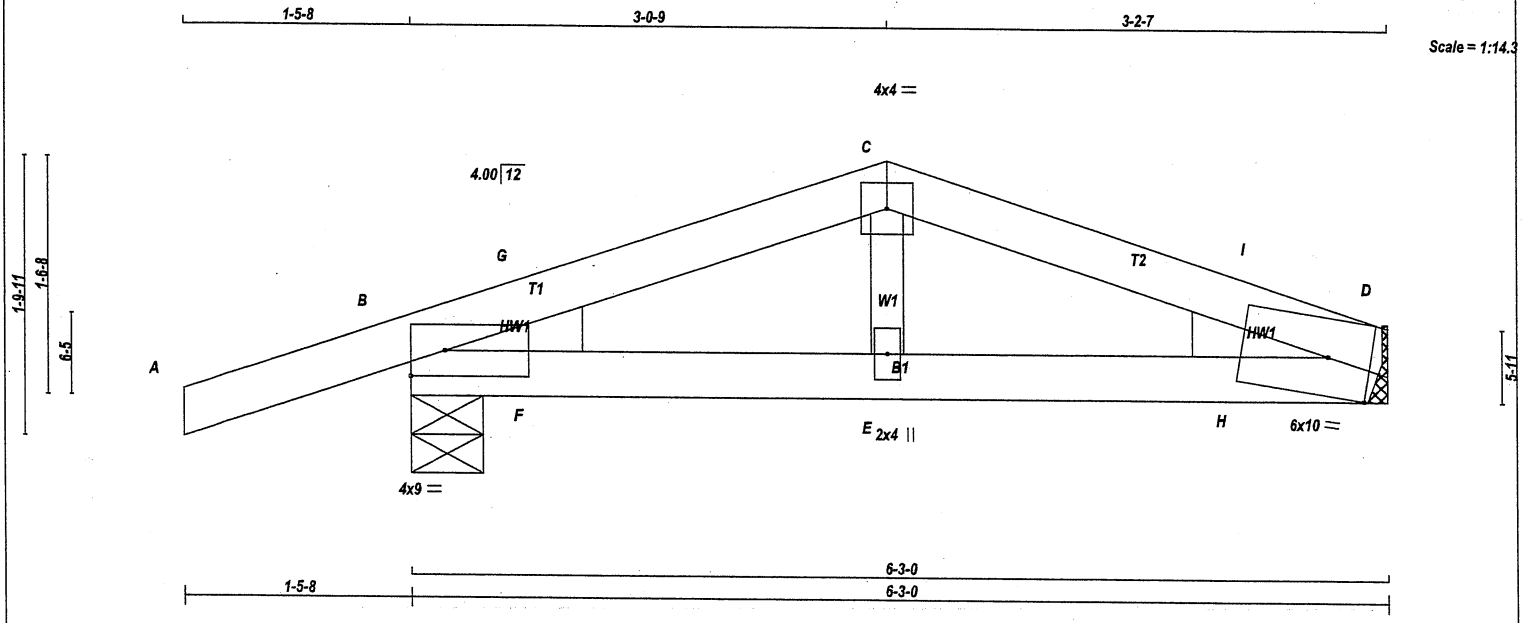

TOTAL WEIGHT = 4 X 19 = 78 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-l	MT20	4.0	9.0		Edge
C	TTW-p	MT20	4.0	4.0		
D	TMBH1-m	MT20	6.0	10.0	Edge	3.25
E	BMW+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
B	483	0	483	0	0	5-8	5-8	2x4 L
D	345	0	345	0	0	MECHANICAL		2x4 R

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	339	237 / 0	0 / 0	0 / 0	0 / 0	102 / 0	0 / 0
D	244	160 / 0	0 / 0	0 / 0	0 / 0	84 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 12	-91.8	-91.8	0.14 (1)	10.00
B- G	-525 / 0	-91.8	-91.8	0.02 (1)	6.25
G- C	-484 / 0	-91.8	-91.8	0.07 (1)	6.25
C- I	-481 / 0	-91.8	-91.8	0.08 (1)	6.25
I- D	-516 / 0	-91.8	-91.8	0.02 (1)	6.25
B- F	0 / 457	-18.5	-18.5	0.12 (1)	10.00
F- E	0 / 457	-18.5	-18.5	0.12 (1)	10.00
E- H	0 / 457	-18.5	-18.5	0.14 (1)	10.00
H- D	0 / 457	-18.5	-18.5	0.14 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- C	0 / 104	0.02 (1)
F- G	-39 / 18	0.00 (1)
H- I	-59 / 12	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	25.6	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.4	PSF
TOTAL	LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.21")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.21")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.14/1.00 (A-B:1) , BC=0.14/1.00 (E-H:1) , WB=0.02/1.00 (C-E:1) , SSI=0.11/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

Structural component only
DWG# T-2136504

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420804	T8CP	2	1	GOLDCOURT DEVELOPMENTS	

Tamarack Roof Truss, Burlington

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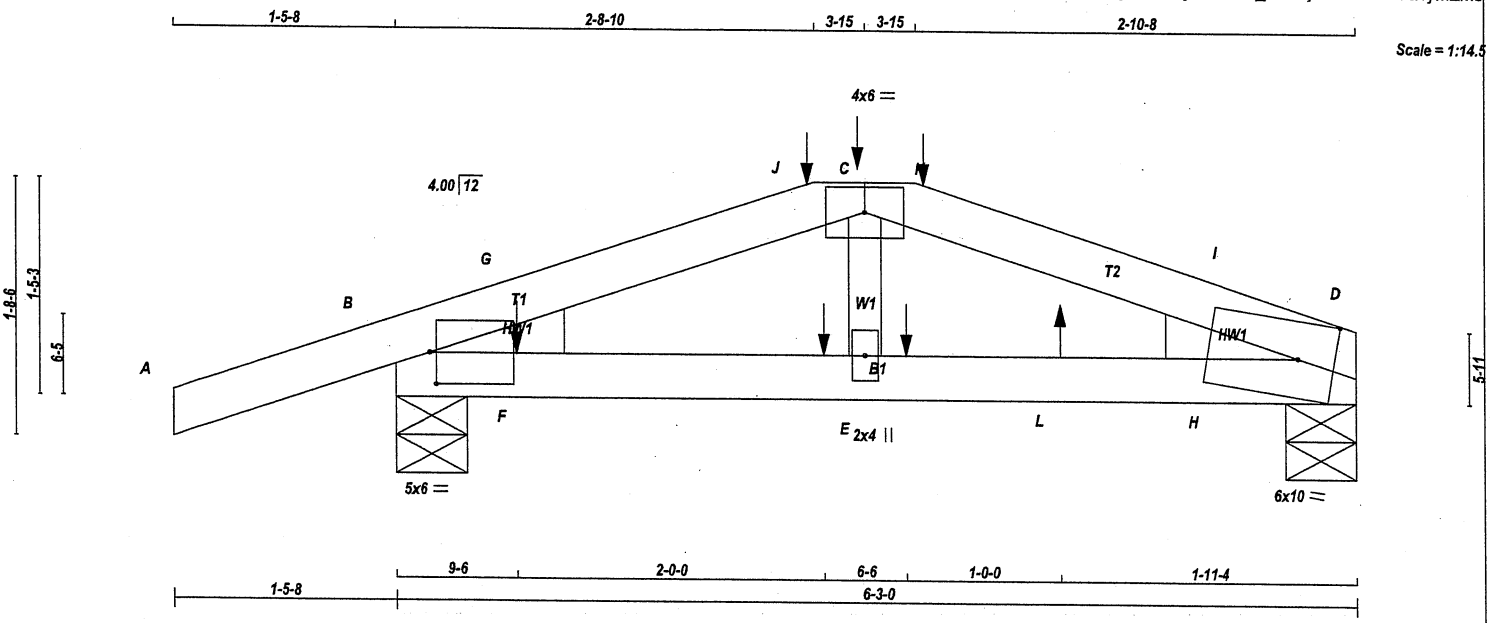

TOTAL WEIGHT = 2 X 19 = 39 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-l	MT20	5.0	6.0	2.50	0.50
C	TTW-p	MT20	4.0	6.0		
D	TMBH1-m	MT20	6.0	10.0	3.00	2.75
E	BMW+w	MT20	2.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
B	660	0	660	0	0	5-8	5-8	2x4 L
D	487	0	487	0	0	5-8	5-8	2x4 R

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	464	320 / 0	0 / 0	0 / 0	0 / 0	144 / 0	0 / 0
D	344	226 / 0	0 / 0	0 / 0	0 / 0	118 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 12	-91.8	-91.8	0.16 (1)	10.00	E- C	0 / 137	0.03 (1)
B- G	-903 / 0	-91.8	-91.8	0.05 (1)	6.25	F- G	0 / 38	0.00 (1)
G- J	-820 / 0	-91.8	-91.8	0.10 (1)	6.25	H- I	-34 / 23	0.00 (1)
J- C	-820 / 0	-91.8	-91.8	0.10 (1)	6.25			
C- K	-818 / 0	-91.8	-91.8	0.11 (1)	6.25			
K- I	-818 / 0	-91.8	-91.8	0.11 (1)	6.25			
I- D	-878 / 0	-91.8	-91.8	0.03 (1)	6.25			
B- F	0 / 784	-18.5	-18.5	0.17 (1)	10.00			
F- E	0 / 784	-18.5	-18.5	0.18 (1)	10.00			
E- L	0 / 784	-18.5	-18.5	0.20 (1)	10.00			
L- H	0 / 784	-18.5	-18.5	0.20 (1)	10.00			
H- D	0 / 784	-18.5	-18.5	0.20 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-0-9	-31	-31	---	FRONT	VERT	TOTAL	---	C1
E	2-9-6	-23	-23	---	FRONT	VERT	TOTAL	---	C1
E	3-3-12	-23	-23	---	FRONT	VERT	TOTAL	---	C1
F	9-6	-25	-25	---	FRONT	VERT	TOTAL	---	C1
J	2-8-10	-64	-64	---	FRONT	VERT	TOTAL	---	C1
K	3-4-8	-64	-64	---	FRONT	VERT	TOTAL	---	C1
L	4-3-12	4	1	4	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.21")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.21")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.16/1.00 (A-B:1) , BC=0.20/1.00 (E-H:1) , WB=0.03/1.00 (C-E:1) , SSI=0.14/1.00 (C-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.61 (B) (INPUT = 0.90)
JSI METAL= 0.18 (B) (INPUT = 1.00)

Structural component only
DWG# T-2136505

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420804	T9	2	1	GOLDCOURT DEVELOPMENTS	

Tamarack Roof Truss, Burlington

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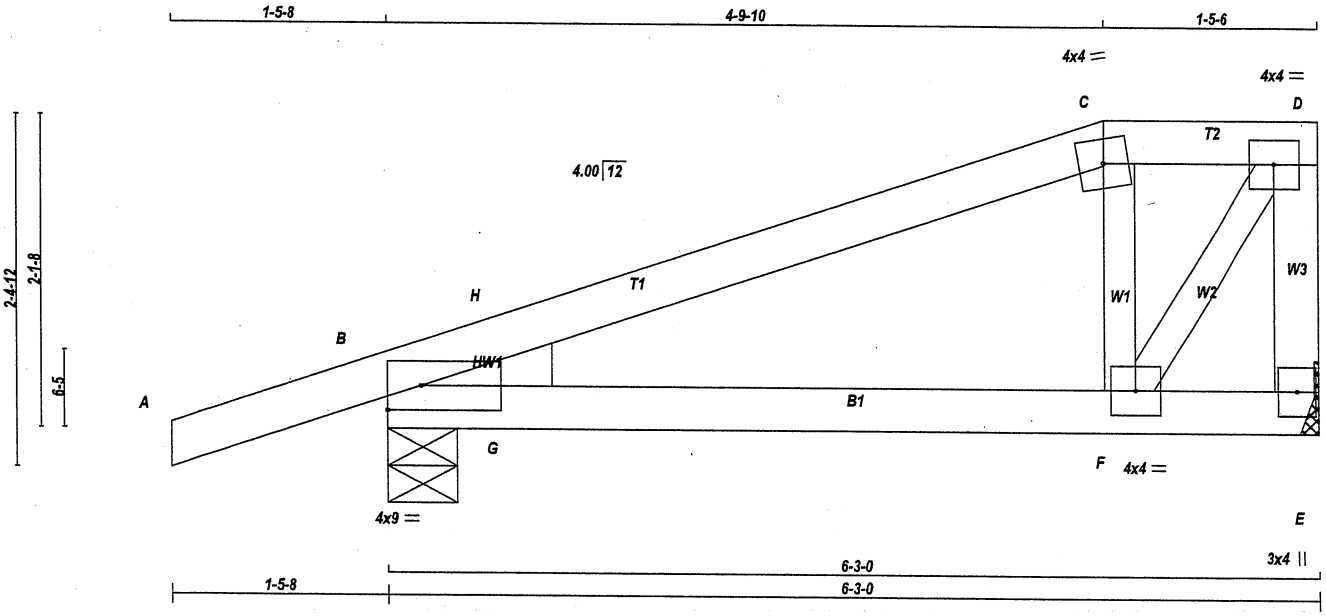

TOTAL WEIGHT = 2 X 22 = 44 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-l	MT20	4.0	9.0		Edge
C	TTW-m	MT20	4.0	4.0		
D	TMVW-t	MT20	4.0	4.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMWW-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
E	345	0	345	0	0	MECHANICAL		
B	483	0	483	0	0	5-8	5-8	2x4 L

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	244	160 / 0	0 / 0	0 / 0	0 / 0	84 / 0	0 / 0
B	339	237 / 0	0 / 0	0 / 0	0 / 0	102 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 12	-91.8	-91.8	0.14 (1)	10.00	F- C	-157 / 0	0.02 (1)
B- H	-288 / 0	-91.8	-91.8	0.03 (1)	6.25	F- D	0 / 424	0.10 (1)
H- C	-275 / 0	-91.8	-91.8	0.23 (1)	6.25	G- H	-185 / 3	0.00 (1)
C- D	-250 / 0	-91.8	-91.8	0.03 (1)	6.25			
E- D	-407 / 0	0.0	0.0	0.05 (1)	7.81			
B- G	0 / 259	-18.5	-18.5	0.23 (1)	10.00			
G- F	0 / 259	-18.5	-18.5	0.23 (1)	10.00			
F- E	0 / 0	-18.5	-18.5	0.14 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.21")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.21")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.23/1.00 (C-H:1) , BC=0.23/1.00 (F-G:1) , WB=0.10/1.00 (D-F:1) , SSI=0.14/1.00 (C-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.44 (F) (INPUT = 0.90)
JSI METAL= 0.12 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
11/04/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2136506

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420804	T10	2	1	GOLDCOURT DEVELOPMENTS	

Tamarack Roof Truss, Burlington

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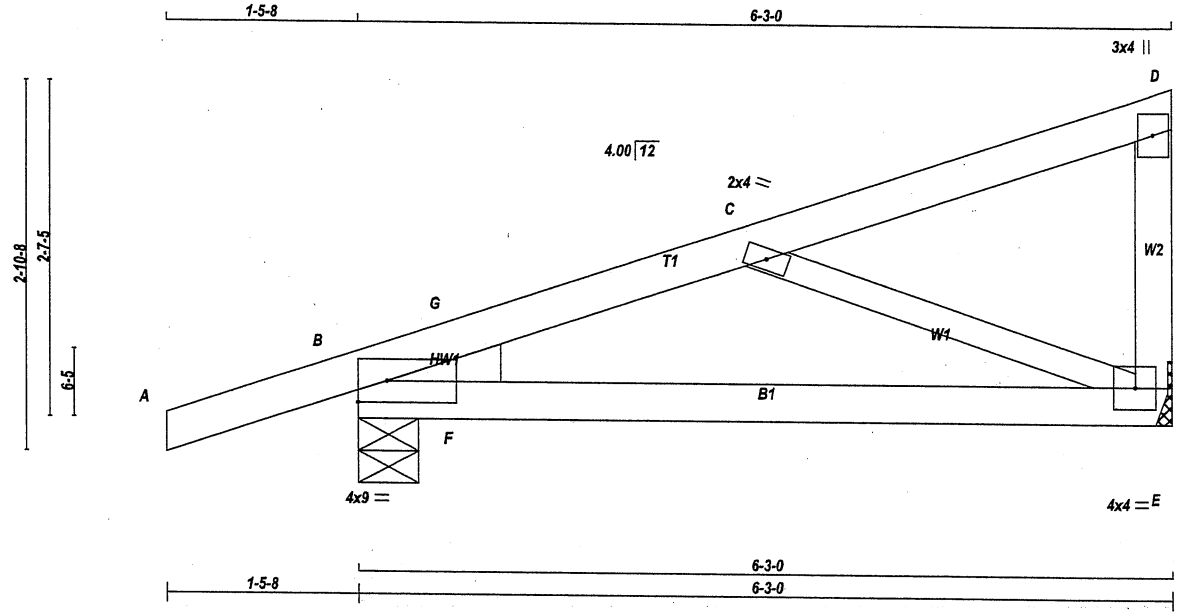

TOTAL WEIGHT = 2 X 22 = 45 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-I	MT20	4.0	9.0		Edge
C	TMW+w	MT20	2.0	4.0		
D	TMV+p	MT20	3.0	4.0		
E	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
E	345	0	345	0	0	MECHANICAL		
B	483	0	483	0	0	5-8	5-8	2x4 L

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	244	160 / 0	0 / 0	0 / 0	0 / 0	84 / 0	0 / 0
B	339	237 / 0	0 / 0	0 / 0	0 / 0	102 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 12	-91.8	-91.8	0.14 (1)	10.00	C- E	-496 / 0	0.11 (1)
B- G	-566 / 0	-91.8	-91.8	0.12 (4)	6.25	F- G	0 / 122	0.00 (1)
G- C	-468 / 0	-91.8	-91.8	0.13 (1)	6.25			
C- D	-9 / 0	-91.8	-91.8	0.13 (1)	10.00			
E- D	-116 / 0	0.0	0.0	0.02 (1)	7.81			
B- F	0 / 461	-18.5	-18.5	0.12 (4)	10.00			
F- E	0 / 461	-18.5	-18.5	0.15 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.21")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.21")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.14/1.00 (A-B:1) , BC=0.15/1.00 (E-F:4) , WB=0.11/1.00 (C-E:1) , SSI=0.13/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.56 (E) (INPUT = 0.90)
JSI METAL= 0.25 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2136507

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420804	T11	2	2	GOLDCOURT DEVELOPMENTS	

Tamarack Roof Truss, Burlington

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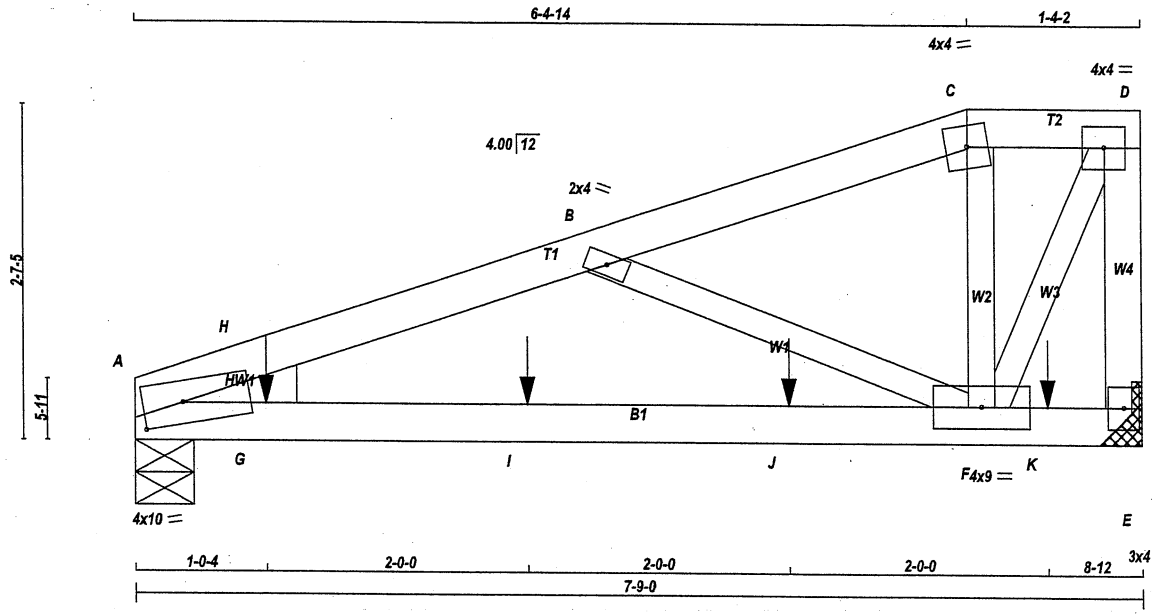

TOTAL WEIGHT = 4 X 27 = 110 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- D	1	12	TOP
D- E	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
A- E	1	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMBH1-m	MT20	4.0	10.0	2.00	3.75
B	TMW+w	MT20	2.0	4.0		
C	TTW-m	MT20	4.0	4.0		
D	TMVW-t	MT20	4.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

LICENSED PROFESSIONAL ENGINEER
11/04/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2136508 1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
E	1103	0	1103	0	0	MECHANICAL		
A	1058	0	1058	0	0	5-8	5-8	2x4 L

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	776	528 / 0	0 / 0	0 / 0	0 / 0	248 / 0	0 / 0
A	745	508 / 0	0 / 0	0 / 0	0 / 0	237 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.08 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- H	-1759 / 0	-91.8	-91.8	0.30 (1)	6.08	B- F	-771 / 0	0.09 (1)
H- B	-1392 / 0	-91.8	-91.8	0.30 (1)	6.25	F- C	0 / 117	0.01 (1)
B- C	-755 / 0	-91.8	-91.8	0.17 (1)	6.25	F- D	0 / 1480	0.18 (1)
C- D	-698 / 0	-91.8	-91.8	0.02 (1)	6.25	G- H	0 / 743	0.00 (1)
E- D	-1364 / 0	0.0	0.0	0.10 (1)	7.81			
A- G	0 / 1390	-18.5	-18.5	0.22 (1)	10.00			
G- I	0 / 1390	-18.5	-18.5	0.50 (1)	10.00			
I- J	0 / 1390	-18.5	-18.5	0.50 (1)	10.00			
J- F	0 / 1390	-18.5	-18.5	0.50 (1)	10.00			
F- K	0 / 0	-18.5	-18.5	0.38 (1)	10.00			
K- E	0 / 0	-18.5	-18.5	0.38 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-0-4	-229	-229	---	BACK	VERT	TOTAL	---	C1
I	3-0-4	-229	-229	---	BACK	VERT	TOTAL	---	C1
J	5-0-4	-229	-229	---	BACK	VERT	TOTAL	---	C1
K	7-0-4	-231	-231	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.26")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.26")
CALCULATED VERT. DEFL.(TL) = L/ 780 (0.12")

CSI: TC=0.30/1.00 (A-H:1) , BC=0.50/1.00 (F-G:1) , WB=0.18/1.00 (D-F:1) , SSI=0.26/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (D) (INPUT = 0.90)
JSI METAL= 0.20 (D) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420804	T11	2	2	GOLDCOURT DEVELOPMENTS	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
F	BMWWW-t	MT20	4.0	9.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2136508

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420804	T12G	8	1	GOLDCOURT DEVELOPMENTS	

Tamarack Roof Truss, Burlington

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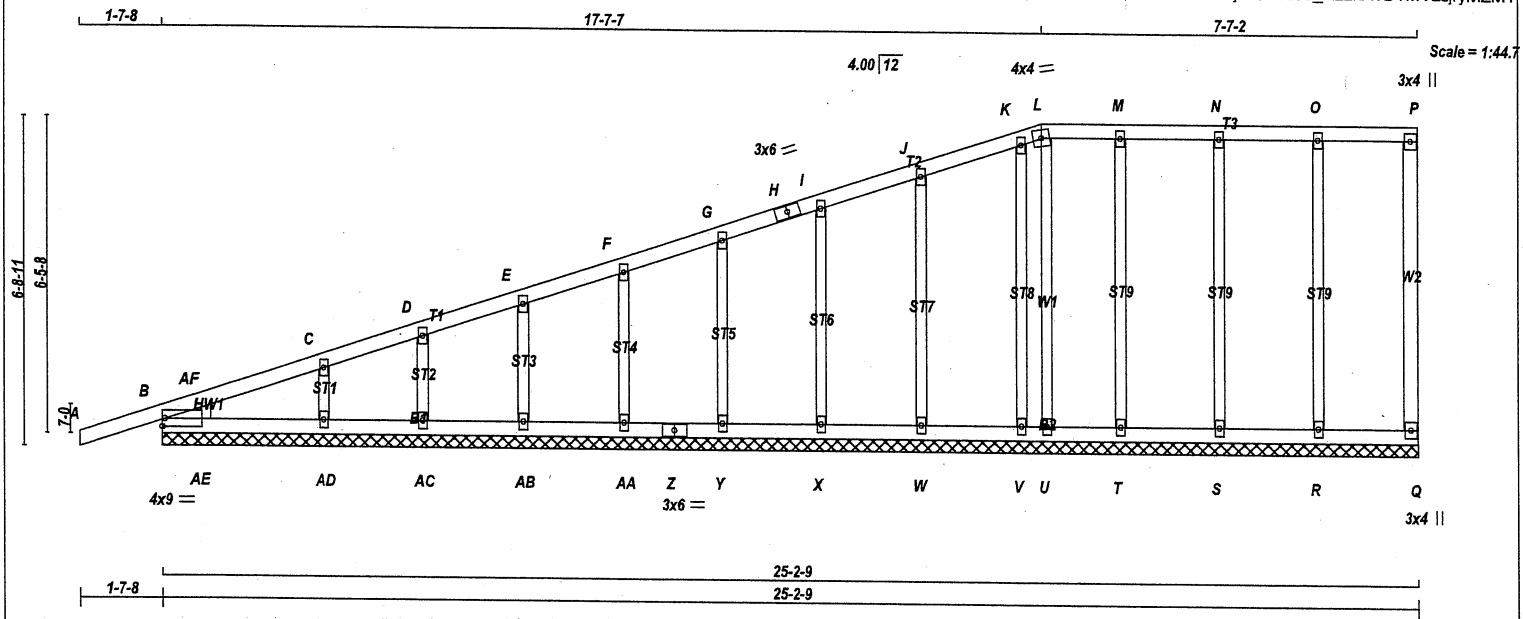

TOTAL WEIGHT = 8 X 110 = 878 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - H	2x4	DRY	No.2	SPF
H - L	2x4	DRY	No.2	SPF
L - P	2x4	DRY	No.2	SPF
Q - P	2x4	DRY	No.2	SPF
B - Z	2x4	DRY	No.2	SPF
Z - Q	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMBH1-I	MT20	4.0	9.0	Edge
C, D, E, F, G, I, J, K, M, N, O					
C	TMW+w	MT20	2.0	4.0	
H	TS-t	MT20	3.0	6.0	
L	TTW-m	MT20	4.0	4.0	
P	TMV+p	MT20	3.0	4.0	
Q	BMV1+p	MT20	3.0	4.0	
R, S, T, U, V, W, X, Y, AA, AB, AC, AD					
R	BMW1+w	MT20	2.0	4.0	
Z	BS-t	MT20	3.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

HEEL
WEDGE
2x4 L
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 12	-91.8	-91.8	0.18 (1)	10.00
B-AF	-46 / 0	-91.8	-91.8	0.03 (4)	6.25
AF- C	-17 / 0	-91.8	-91.8	0.09 (1)	6.25
C- D	-24 / 0	-91.8	-91.8	0.09 (1)	6.25
D- E	-12 / 0	-91.8	-91.8	0.04 (1)	6.25
E- F	-11 / 0	-91.8	-91.8	0.04 (1)	6.25
F- G	-8 / 0	-91.8	-91.8	0.04 (1)	10.00
G- H	-6 / 0	-91.8	-91.8	0.04 (1)	10.00
H- I	-6 / 0	-91.8	-91.8	0.04 (1)	10.00
I- J	-3 / 0	-91.8	-91.8	0.05 (1)	10.00
J- K	-5 / 0	-91.8	-91.8	0.05 (1)	10.00
K- L	-10 / 0	-91.8	-91.8	0.02 (1)	6.25
L- M	0 / 0	-91.8	-91.8	0.03 (1)	10.00
M- N	0 / 0	-91.8	-91.8	0.04 (1)	10.00
N- O	0 / 0	-91.8	-91.8	0.05 (1)	10.00
O- P	0 / 0	-91.8	-91.8	0.05 (1)	10.00
Q- P	-75 / 0	0.0	0.0	0.06 (1)	7.81
B-AE	0 / 26	-18.5	-18.5	0.07 (1)	10.00
AE-AD	0 / 26	-18.5	-18.5	0.07 (1)	10.00
AD-AC	0 / 17	-18.5	-18.5	0.05 (1)	10.00
AC-AB	0 / 13	-18.5	-18.5	0.02 (4)	10.00
AB-AA	0 / 10	-18.5	-18.5	0.02 (4)	10.00
AA- Z	0 / 7	-18.5	-18.5	0.01 (4)	10.00
Z- Y	0 / 7	-18.5	-18.5	0.01 (4)	10.00
Y- X	0 / 5	-18.5	-18.5	0.01 (4)	10.00
X- W	0 / 4	-18.5	-18.5	0.02 (4)	10.00
W- V	0 / 2	-18.5	-18.5	0.02 (4)	10.00
V- U	0 / 0	-18.5	-18.5	0.01 (4)	10.00
U- T	0 / 0	-18.5	-18.5	0.01 (4)	10.00
T- S	0 / 0	-18.5	-18.5	0.01 (4)	10.00
S- R	0 / 0	-18.5	-18.5	0.02 (4)	10.00
R- Q	0 / 0	-18.5	-18.5	0.02 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R- O	-202 / 0	0.14 (1)
S- N	-185 / 0	0.12 (1)
T- M	-178 / 0	0.12 (1)
V- K	-131 / 0	0.08 (1)
W- J	-195 / 0	0.10 (1)
X- I	-181 / 0	0.07 (1)
Y- G	-183 / 0	0.05 (1)
AA- F	-181 / 0	0.04 (1)
AB- E	-189 / 0	0.03 (1)
AC- D	-156 / 0	0.02 (1)
AD- C	-257 / 0	0.04 (1)
U- L	-49 / 0	0.03 (1)
AE-AF	-79 / 4	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.18/1.00 (A-B:1) , BC=0.07/1.00 (B-AE:1) , WB=0.14/1.00 (O-R:1) , SSI=0.12/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.58 (L) (INPUT = 0.90)
JSI METAL= 0.08 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
11/04/21
H.J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2136509

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420804	T13	2	1	GOLDCOURT DEVELOPMENTS	

Tamarack Roof Truss, Burlington

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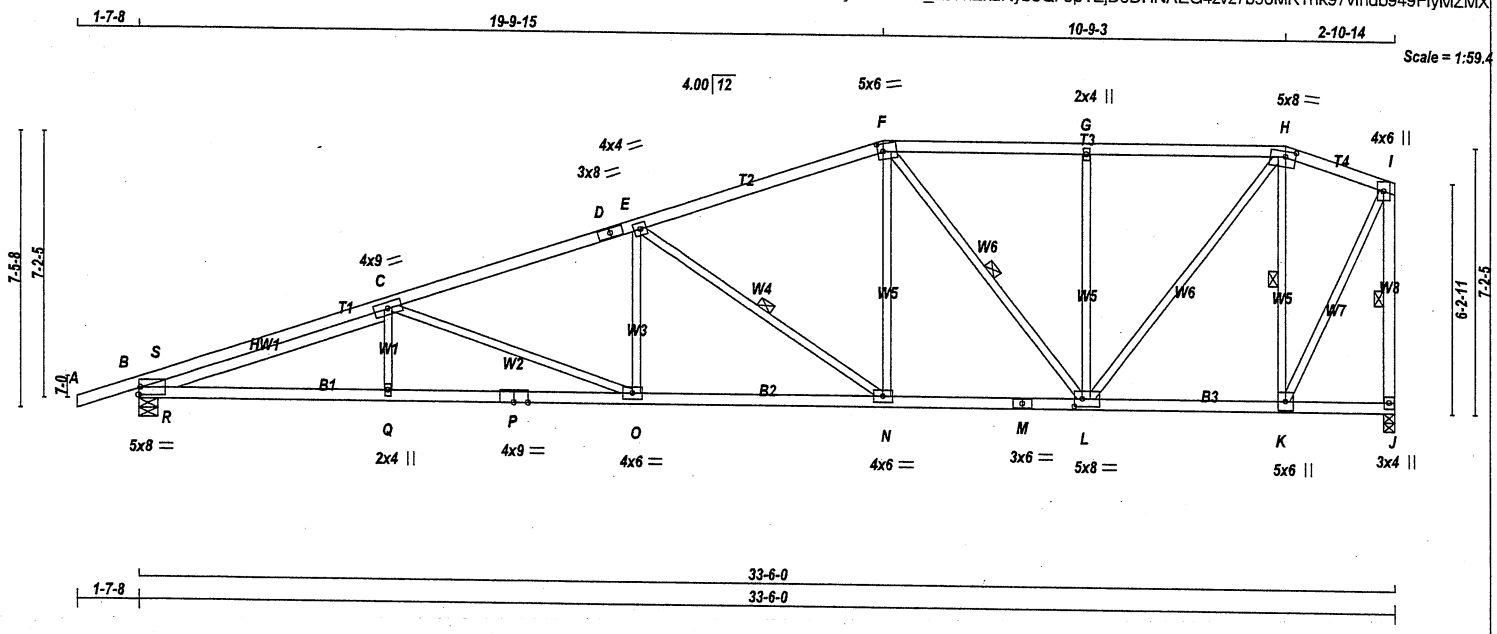

TOTAL WEIGHT = 2 X 146 = 293 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - I	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
B - P	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
REINFORCING MEMBERS				
HW1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBW1-I	MT20	5.0	8.0		Edge
C	TMWWW-t	MT20	4.0	9.0		
D	TS-t	MT20	3.0	8.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.50	1.75
G	TMW+w	MT20	2.0	4.0		
H	TTWW-m	MT20	5.0	8.0	1.75	3.25
I	TMVW+p	MT20	4.0	6.0		
J	BMV1+p	MT20	3.0	4.0		
K	BMWW+t	MT20	5.0	6.0		
L	BMWWW-t	MT20	5.0	8.0	2.50	2.50
M	BS-t	MT20	3.0	6.0		
N	BMWW-t	MT20	4.0	6.0		
O	BMWW-t	MT20	4.0	6.0		
P	BS-t	MT20	4.0	9.0		
Q	BMW+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2136510

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	2000	0	2000	0	0	5-8	5-8
J	1847	0	1847	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	1412	943 / 0	0 / 0	0 / 0	0 / 0	469 / 0	0 / 0
J	1306	857 / 0	0 / 0	0 / 0	0 / 0	449 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.22 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-N, F-L, H-K, I-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI	MAX (LC)	MAX. UNBRAC LENGTH
A- B	0 / 12	-91.8	-91.8	0.18 (1)		10.00
B- S	-3260 / 0	-91.8	-91.8	0.30 (1)		3.65
S- C	-2896 / 0	-91.8	-91.8	0.46 (1)		3.74
C- D	-3417 / 0	-91.8	-91.8	0.70 (1)		3.22
D- E	-3417 / 0	-91.8	-91.8	0.70 (1)		3.22
E- F	-2299 / 0	-91.8	-91.8	0.59 (1)		3.91
F- G	-1720 / 0	-91.8	-91.8	0.38 (1)		4.68
G- H	-1720 / 0	-91.8	-91.8	0.38 (1)		4.68
H- I	-777 / 0	-91.8	-91.8	0.11 (1)		6.25
J- I	-1831 / 0	0.0	0.0	0.34 (1)		4.95
B- R	0 / 2752	-18.5	-18.5	0.49 (1)		10.00
R- Q	0 / 4050	-18.5	-18.5	0.77 (1)		10.00
Q- P	0 / 4046	-18.5	-18.5	0.73 (1)		10.00
P- O	0 / 4046	-18.5	-18.5	0.73 (1)		10.00
O- N	0 / 3249	-18.5	-18.5	0.60 (1)		10.00
N- M	0 / 2175	-18.5	-18.5	0.42 (1)		10.00
M- L	0 / 2175	-18.5	-18.5	0.42 (1)		10.00
L- K	0 / 717	-18.5	-18.5	0.16 (1)		10.00
K- J	0 / 0	-18.5	-18.5	0.08 (4)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	0 / 133	0.05 (4)
C- O	-855 / 0	0.82 (1)
O- E	0 / 436	0.10 (1)
E- N	-1327 / 0	0.60 (1)
N- F	0 / 876	0.20 (1)
F- L	-741 / 0	0.38 (1)
L- G	-609 / 0	0.55 (1)
L- H	0 / 1630	0.37 (1)
K- H	-1361 / 0	0.44 (1)
K- I	0 / 1627	0.37 (1)
R- S	0 / 547	0.00 (1)
R- C	-1408 / 0	0.87 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.12")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.21")
ALLOWABLE DEFL.(TL)= L/360 (1.12")
CALCULATED VERT. DEFL.(TL) = L/ 985 (0.41")

CSI: TC=0.70/1.00 (C-E:1) , BC=0.77/1.00 (Q-R:1) , WB=0.87/1.00 (C-R:1) , SSI=0.27/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.94 (P) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420804	T14	2	1	GOLDCOURT DEVELOPMENTS	

Tamarack Roof Truss, Burlington

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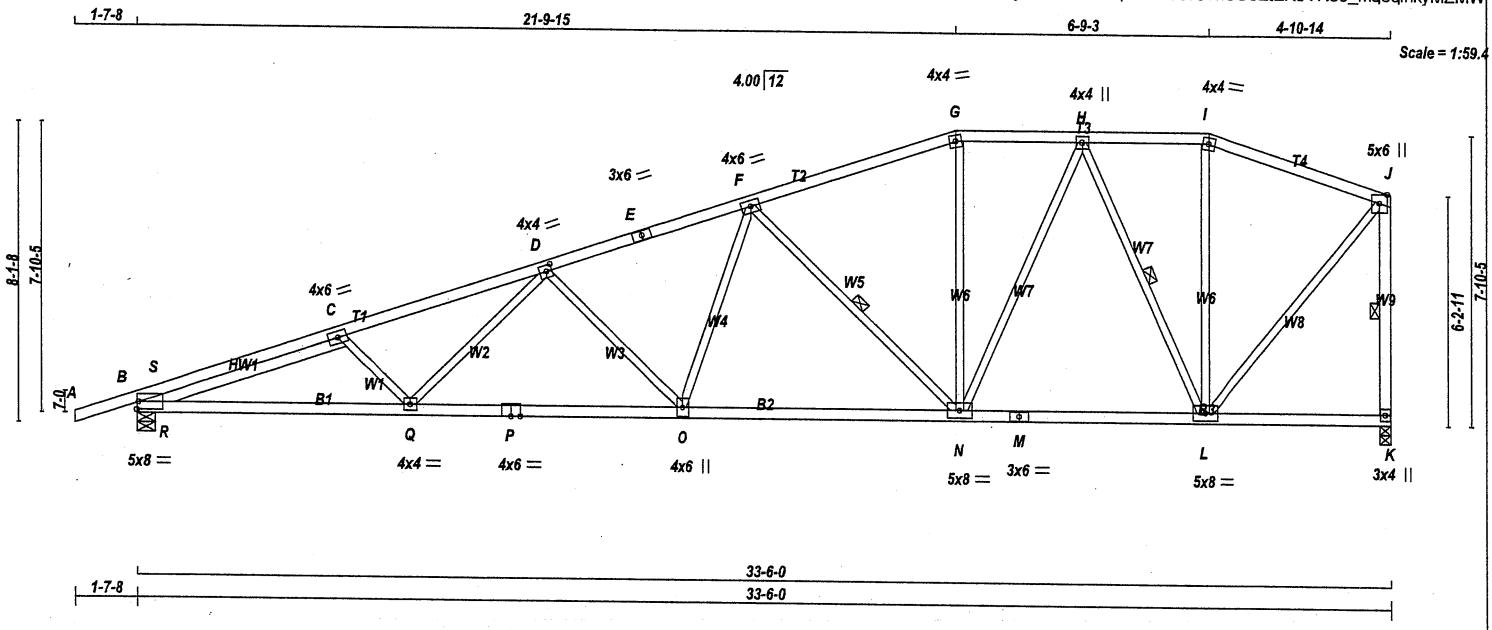

TOTAL WEIGHT = 2 X 146 = 291 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - J	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
B - P	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
REINFORCING MEMBERS				
HW1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBW1-I	MT20	5.0	8.0		Edge
C	TMWW-t	MT20	4.0	6.0		
D	TMWW-t	MT20	4.0	4.0	2.00	1.75
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	6.0		
G	TTW-m	MT20	4.0	4.0		
H	TMWW+t	MT20	4.0	4.0		
I	TTW-m	MT20	4.0	4.0		
J	TMVW+p	MT20	5.0	6.0	Edge	
K	BMV1+p	MT20	3.0	4.0		
L	BMWWW-t	MT20	5.0	8.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	5.0	8.0		
O	BMWW+t	MT20	4.0	6.0		
P	BS-t	MT20	4.0	6.0		
Q	BMWW-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	2000	0	2000	0	0	5-8	5-8
K	1847	0	1847	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	1412	943 / 0	0 / 0	0 / 0	0 / 0	469 / 0	0 / 0
K	1306	857 / 0	0 / 0	0 / 0	0 / 0	449 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.18 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-N, J-K, H-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 12	-91.8	-91.8	0.18 (1)	10.00
B- S	-3177 / 0	-91.8	-91.8	0.20 (1)	3.80
S- C	-2759 / 0	-91.8	-91.8	0.31 (1)	3.95
C- D	-4065 / 0	-91.8	-91.8	0.52 (1)	3.18
D- E	-3119 / 0	-91.8	-91.8	0.46 (1)	3.59
E- F	-3119 / 0	-91.8	-91.8	0.46 (1)	3.59
F- G	-1970 / 0	-91.8	-91.8	0.37 (1)	4.45
G- H	-1857 / 0	-91.8	-91.8	0.16 (1)	4.79
H- I	-1046 / 0	-91.8	-91.8	0.14 (1)	6.00
I- J	-1103 / 0	-91.8	-91.8	0.30 (1)	5.66
K- J	-1811 / 0	0.0	0.0	0.34 (1)	4.97
B- R	0 / 2624	-18.5	-18.5	0.43 (1)	10.00
R- Q	0 / 4075	-18.5	-18.5	0.76 (1)	10.00
Q- P	0 / 3499	-18.5	-18.5	0.66 (1)	10.00
P- O	0 / 3499	-18.5	-18.5	0.66 (1)	10.00
O- N	0 / 2736	-18.5	-18.5	0.54 (1)	10.00
N- M	0 / 1533	-18.5	-18.5	0.35 (1)	10.00
M- L	0 / 1533	-18.5	-18.5	0.35 (1)	10.00
L- K	0 / 0	-18.5	-18.5	0.14 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- Q	-334 / 0	0.06 (1)
Q- D	0 / 528	0.12 (1)
D- O	-790 / 0	0.39 (1)
O- F	0 / 754	0.17 (1)
F- N	-1270 / 0	0.55 (1)
N- G	0 / 298	0.07 (1)
L- I	0 / 57	0.02 (4)
L- J	0 / 1620	0.36 (1)
N- H	0 / 815	0.18 (1)
H- L	-1226 / 0	0.57 (1)
R- S	0 / 689	0.00 (1)
R- C	-1587 / 0	0.58 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.12")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.20")
ALLOWABLE DEFL.(TL)= L/360 (1.12")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.40")

CSI: TC=0.52/1.00 (C-D:1) , BC=0.76/1.00 (Q-R:1) , WB=0.58/1.00 (C-R:1) , SSI=0.22/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (P) (INPUT = 0.90)
JSI METAL= 0.82 (P) (INPUT = 1.00)

Structural component only
DWG# T-2136511

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420804	T15	2	1	GOLDCOURT DEVELOPMENTS	

Tamarack Roof Truss, Burlington

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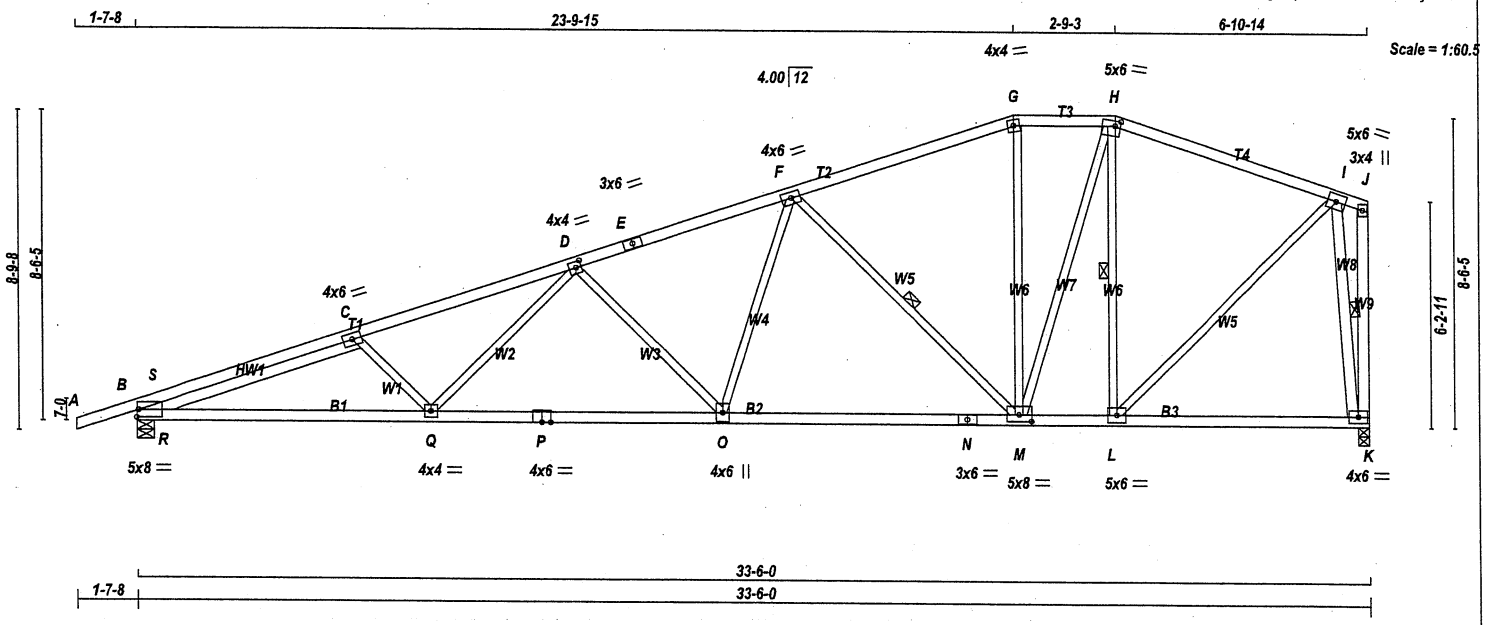


TOTAL WEIGHT = 2 X 151 = 302 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
B - P	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
REINFORCING MEMBERS				
HW1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBW1-I	MT20	5.0	8.0		Edge
C	TMWW-t	MT20	4.0	6.0		
D	TMWW-t	MT20	4.0	4.0	2.00	1.75
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	6.0		
G	TTW-m	MT20	4.0	4.0		
H	TTWW-m	MT20	5.0	6.0	1.75	1.75
I	TMWW-t	MT20	5.0	6.0		
J	TMV+p	MT20	3.0	4.0		
K	BMVW1-t	MT20	4.0	6.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMWWW-t	MT20	5.0	8.0	2.25	4.00
N	BS-t	MT20	3.0	6.0		
O	BMWW+t	MT20	4.0	6.0		
P	BS-t	MT20	4.0	6.0		
Q	BMWW-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2136512

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	2000	0	2000	0	0	5-8	5-8
K	1847	0	1847	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	1412	943 / 0	0 / 0	0 / 0	0 / 0	469 / 0	0 / 0
K	1306	857 / 0	0 / 0	0 / 0	0 / 0	449 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.16 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-M, H-L, I-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 12	-91.8	-91.8	0.18 (1)	10.00
B- S	-3239 / 0	-91.8	-91.8	0.23 (1)	3.74
S- C	-2827 / 0	-91.8	-91.8	0.39 (1)	3.84
C- D	-4000 / 0	-91.8	-91.8	0.59 (1)	3.16
D- E	-2919 / 0	-91.8	-91.8	0.52 (1)	3.64
E- F	-2919 / 0	-91.8	-91.8	0.52 (1)	3.64
F- G	-1637 / 0	-91.8	-91.8	0.44 (1)	4.70
G- H	-1538 / 0	-91.8	-91.8	0.11 (1)	5.21
H- I	-1337 / 0	-91.8	-91.8	0.42 (1)	5.11
I- J	0 / 123	-91.8	-91.8	0.47 (1)	10.00
K- J	0 / 350	0.0	0.0	0.06 (1)	10.00
B- R	0 / 2689	-18.5	-18.5	0.43 (1)	10.00
R- Q	0 / 4065	-18.5	-18.5	0.77 (1)	10.00
Q- P	0 / 3370	-18.5	-18.5	0.68 (1)	10.00
P- O	0 / 3370	-18.5	-18.5	0.68 (1)	10.00
O- N	0 / 2534	-18.5	-18.5	0.55 (1)	10.00
N- M	0 / 2534	-18.5	-18.5	0.55 (1)	10.00
M- L	0 / 1241	-18.5	-18.5	0.32 (1)	10.00
L- K	0 / 293	-18.5	-18.5	0.21 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- Q	-412 / 0	0.08 (1)
Q- D	0 / 624	0.14 (1)
D- O	-874 / 0	0.53 (1)
O- F	0 / 833	0.19 (1)
F- M	-1416 / 0	0.75 (1)
M- G	0 / 164	0.04 (4)
M- H	0 / 966	0.22 (1)
L- H	-851 / 0	0.39 (1)
L- I	0 / 1361	0.31 (1)
I- K	-2164 / 0	0.42 (1)
R- S	0 / 661	0.00 (1)
R- C	-1499 / 0	0.69 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.12")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.20")
ALLOWABLE DEFL.(TL)= L/360 (1.12")
CALCULATED VERT. DEFL.(TL) = L/ 977 (0.41")

CSI: TC=0.59/1.00 (C-D:1) , BC=0.77/1.00 (Q-R:1) , WB=0.75/1.00 (F-M:1) , SSI=0.32/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

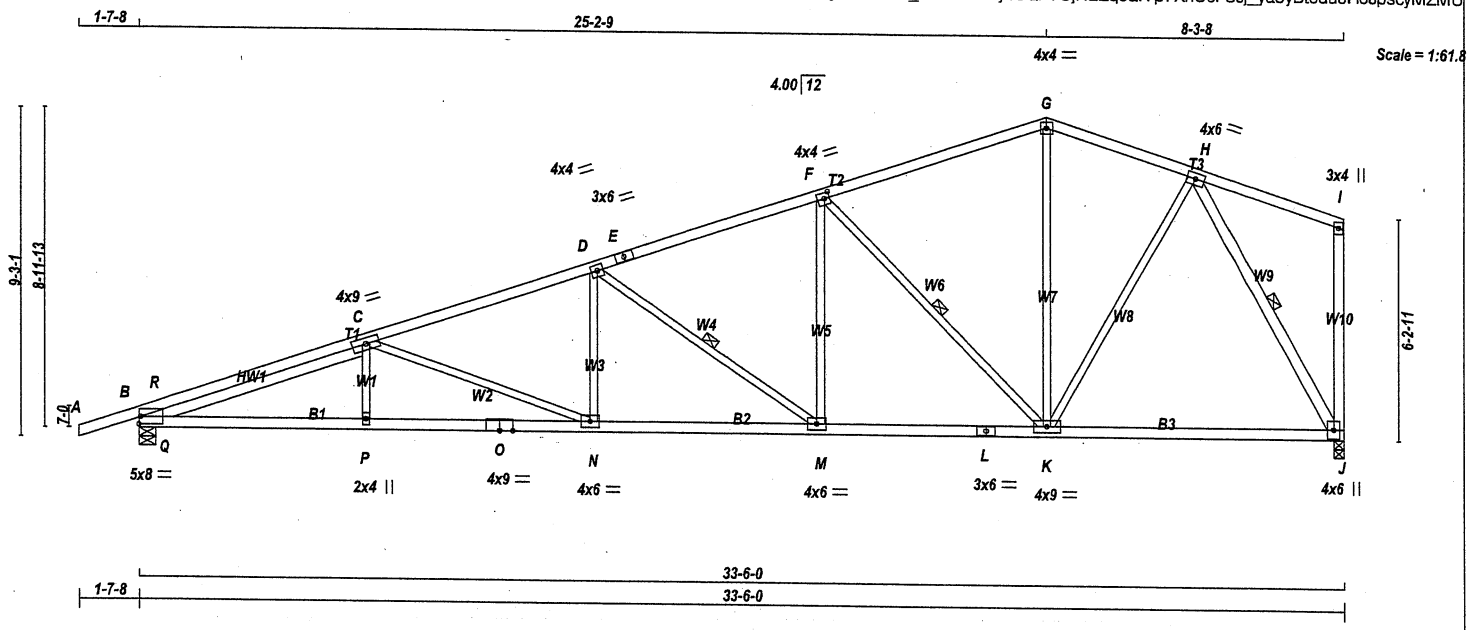
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (B) (INPUT = 0.90)
JSI METAL= 0.80 (P) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420804	T16	8	1	GOLDCOURT DEVELOPMENTS	

Tamarack Roof Truss, Burlington



TOTAL WEIGHT = 8 X 146 = 1167 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
B - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
REINFORCING MEMBERS				
HW1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBW1-I	MT20	5.0	8.0		Edge
C	TMWWW-t	MT20	4.0	9.0		
D	TMWW-t	MT20	4.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0	2.00	1.75
G	TTW-p	MT20	4.0	4.0		
H	TMWW-t	MT20	4.0	6.0		
I	TMV+p	MT20	3.0	4.0		
J	BMVW1+p	MT20	4.0	6.0		
K	BMWWW-t	MT20	4.0	9.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	6.0		
N	BMWW-t	MT20	4.0	6.0		
O	BS-t	MT20	4.0	9.0		
P	BMW+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	2000	0	2000	0	0	5-8	5-8
J	1847	0	1847	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	1412	943 / 0	0 / 0	0 / 0	0 / 0	469 / 0	0 / 0
J	1306	857 / 0	0 / 0	0 / 0	0 / 0	449 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.39 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-M, F-K, H-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 12	-91.8	-91.8	0.18 (1)	10.00
B- R	-3219 / 0	-91.8	-91.8	0.27 (1)	3.71
R- C	-2838 / 0	-91.8	-91.8	0.43 (1)	3.80
C- D	-3484 / 0	-91.8	-91.8	0.56 (1)	3.39
D- E	-2493 / 0	-91.8	-91.8	0.54 (1)	3.87
E- F	-2493 / 0	-91.8	-91.8	0.54 (1)	3.87
F- G	-1431 / 0	-91.8	-91.8	0.47 (1)	4.91
G- H	-1427 / 0	-91.8	-91.8	0.22 (1)	5.24
H- I	0 / 14	-91.8	-91.8	0.26 (1)	10.00
J- I	-147 / 0	0.0	0.0	0.12 (1)	7.81
B- Q	0 / 2698	-18.5	-18.5	0.48 (1)	10.00
Q- P	0 / 4073	-18.5	-18.5	0.76 (1)	10.00
P- O	0 / 4069	-18.5	-18.5	0.73 (1)	10.00
O- N	0 / 4069	-18.5	-18.5	0.73 (1)	10.00
N- M	0 / 3306	-18.5	-18.5	0.61 (1)	10.00
M- L	0 / 2370	-18.5	-18.5	0.54 (1)	10.00
L- K	0 / 2370	-18.5	-18.5	0.54 (1)	10.00
K- J	0 / 935	-18.5	-18.5	0.36 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- C	0 / 126	0.04 (4)
C- N	-819 / 0	0.70 (1)
N- D	0 / 422	0.10 (1)
D- M	-1151 / 0	0.46 (1)
M- F	0 / 768	0.17 (1)
F- K	-1491 / 0	0.86 (1)
K- G	0 / 521	0.12 (1)
K- H	0 / 827	0.19 (1)
H- J	-1885 / 0	0.63 (1)
Q- R	0 / 589	0.00 (1)
Q- C	-1494 / 0	0.82 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.12")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.20")
ALLOWABLE DEFL.(TL)= L/360 (1.12")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.39")

CSI: TC=0.56/1.00 (C-D:1) , BC=0.76/1.00 (P-Q:1) , WB=0.86/1.00 (F-K:1) , SSI=0.26/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (F) (INPUT = 0.90)
JSI METAL= 0.93 (O) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100089024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2136513

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420804	T17	4	1	GOLDCOURT DEVELOPMENTS	

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Nov 4 14:30:56 2021 Page 1
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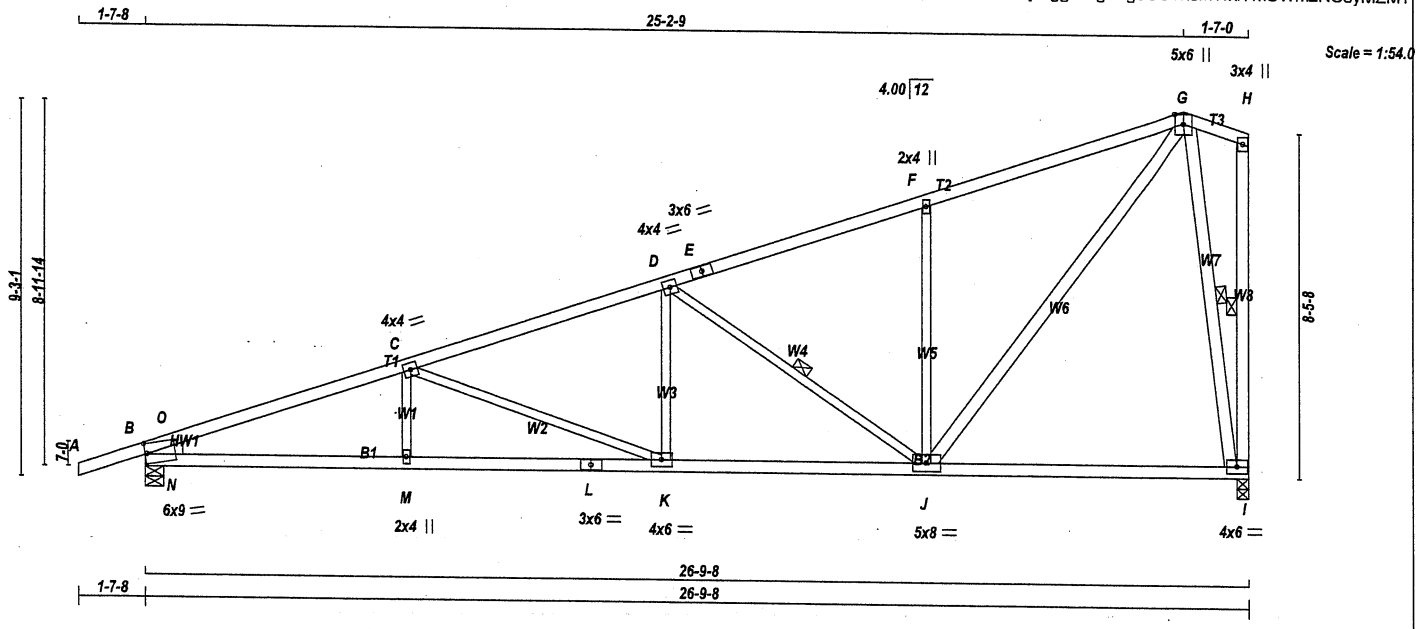


TOTAL WEIGHT = 4 X 118 = 473 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
B - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-m	MT20	6.0	9.0	3.00	0.50
C	TMWW-t	MT20	4.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TTWW+p	MT20	5.0	6.0	Edge	
H	TMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	4.0	6.0		
J	BMWWW-t	MT20	5.0	8.0		
K	BMWW-t	MT20	4.0	6.0		
L	BS-t	MT20	3.0	6.0		
M	BMW+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
B	1630	0	1630	0	0	5-8	5-8	2x4 L
I	1477	0	1477	0	0	3-8	3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	1150	771 / 0	0 / 0	0 / 0	0 / 0	379 / 0	0 / 0
I	1044	685 / 0	0 / 0	0 / 0	0 / 0	359 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.27 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-J, H-I, G-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 12	-91.8	-91.8	0.18 (1)	10.00
B- O	-3477 / 0	-91.8	-91.8	0.52 (1)	3.27
O- C	-3212 / 0	-91.8	-91.8	0.58 (1)	3.42
C- D	-2393 / 0	-91.8	-91.8	0.46 (1)	4.05
D- E	-1386 / 0	-91.8	-91.8	0.43 (1)	5.03
E- F	-1386 / 0	-91.8	-91.8	0.43 (1)	5.03
F- G	-1403 / 0	-91.8	-91.8	0.47 (1)	4.94
G- H	0 / 0	-91.8	-91.8	0.04 (1)	10.00
I- H	-73 / 0	0.0	0.0	0.02 (1)	6.25
B- N	0 / 3038	-18.5	-18.5	0.77 (1)	10.00
N- M	0 / 3038	-18.5	-18.5	0.77 (1)	10.00
M- L	0 / 3038	-18.5	-18.5	0.62 (1)	10.00
L- K	0 / 3038	-18.5	-18.5	0.62 (1)	10.00
K- J	0 / 2276	-18.5	-18.5	0.50 (1)	10.00
J- I	0 / 245	-18.5	-18.5	0.27 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- K	-816 / 0	0.69 (1)
K- D	0 / 420	0.09 (1)
D- J	-1177 / 0	0.47 (1)
J- F	-644 / 0	0.52 (1)
J- G	0 / 1823	0.29 (1)
G- I	-1366 / 0	0.52 (1)
N- O	0 / 407	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.89")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (0.89")
CALCULATED VERT. DEFL.(TL) = L/ 909 (0.35")

CSI: TC=0.58/1.00 (C-O:1) , BC=0.77/1.00 (B-N:1) , WB=0.69/1.00 (C-K:1) , SSI=0.34/1.00 (B-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

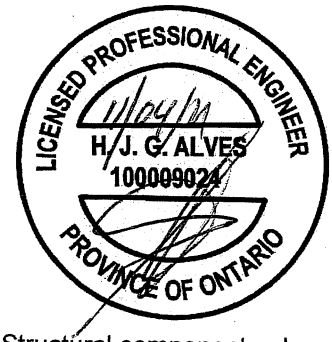
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (J) (INPUT = 0.90)
JSI METAL= 0.97 (L) (INPUT = 1.00)



Structural component only
DWG# T-2136514

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420804	T18	14	1	GOLDCOURT DEVELOPMENTS	

Tamarack Roof Truss, Burlington

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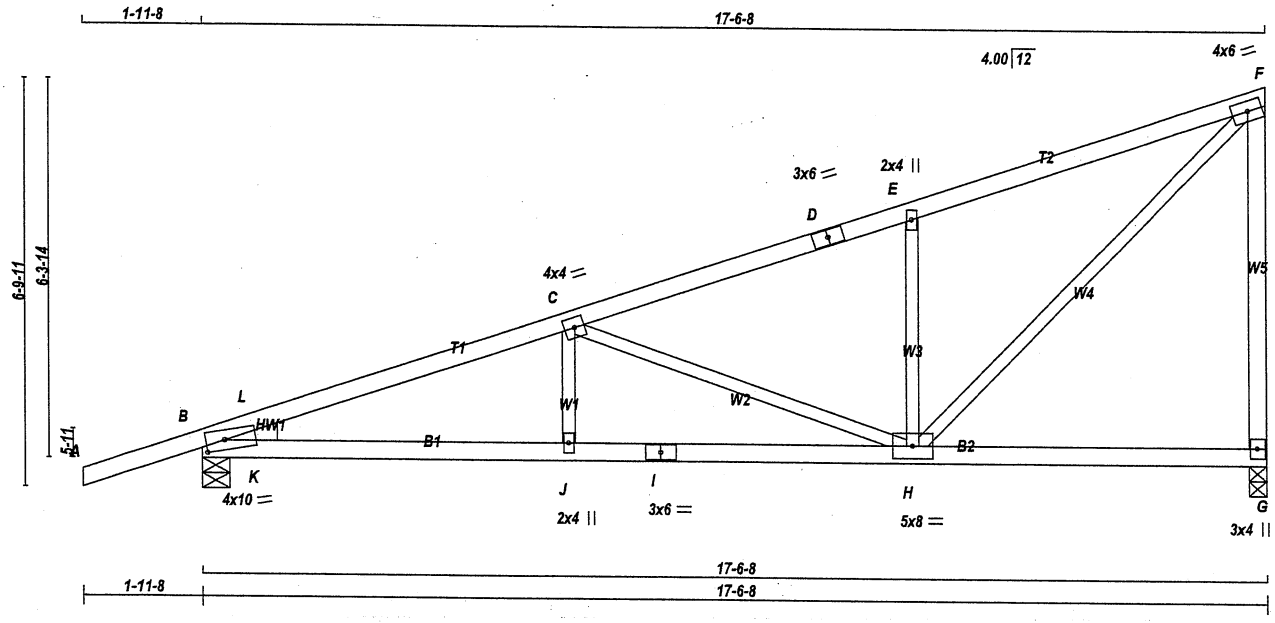

TOTAL WEIGHT = 14 X 67 = 936 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
B - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-m	MT20	4.0	10.0	2.00	3.75
C	TMWW-t	MT20	4.0	4.0		
D	TS-t	MT20	3.0	6.0		
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BS-t	MT20	3.0	6.0		
J	BMW+w	MT20	2.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
G	967	0	967	0	0	3-8	3-8	
B	1151	0	1151	0	0	5-8	5-8	2x4 L

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	684	449 / 0	0 / 0	0 / 0	0 / 0	235 / 0	0 / 0
B	810	551 / 0	0 / 0	0 / 0	0 / 0	259 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.58 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 22	-91.8	-91.8	0.25 (1)	10.00
B- L	-2050 / 0	-91.8	-91.8	0.14 (1)	4.63
L- C	-1953 / 0	-91.8	-91.8	0.28 (1)	4.58
C- D	-1071 / 0	-91.8	-91.8	0.35 (1)	5.65
D- E	-1071 / 0	-91.8	-91.8	0.35 (1)	5.65
E- F	-1087 / 0	-91.8	-91.8	0.38 (1)	5.57
G- F	-922 / 0	0.0	0.0	0.73 (1)	7.81
B- K	0 / 1859	-18.5	-18.5	0.36 (1)	10.00
K- J	0 / 1859	-18.5	-18.5	0.40 (1)	10.00
J- I	0 / 1859	-18.5	-18.5	0.37 (1)	10.00
I- H	0 / 1859	-18.5	-18.5	0.37 (1)	10.00
H- G	0 / 0	-18.5	-18.5	0.15 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- C	0 / 113	0.04 (4)
C- H	-898 / 0	0.58 (1)
H- E	-601 / 0	0.17 (1)
H- F	0 / 1455	0.33 (1)
K- L	-8 / 67	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.58")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.58")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.73/1.00 (F-G:1) , BC=0.40/1.00 (J-K:1) , WB=0.58/1.00 (C-H:1) , SSI=0.24/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (F) (INPUT = 0.90)
JSI METAL= 0.57 (I) (INPUT = 1.00)

Structural component only
DWG# T-2136515

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420804	T19	4	1	GOLDCOURT DEVELOPMENTS	

Tamarack Roof Truss, Burlington

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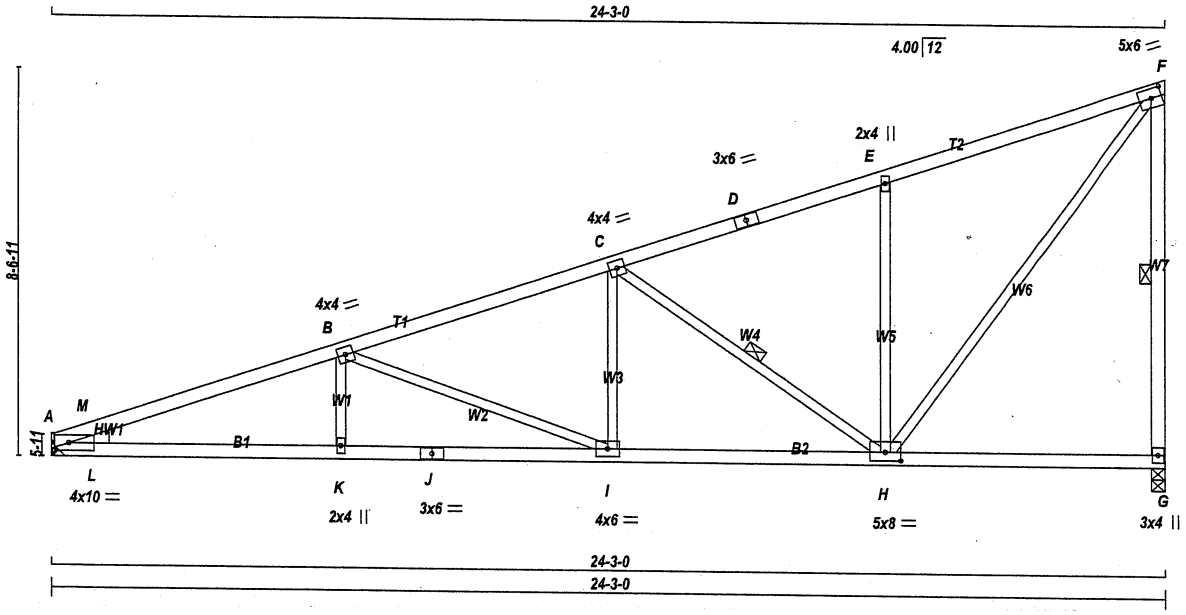

TOTAL WEIGHT = 4 X 95 = 381 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
A - J	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMBH1-I	MT20	4.0	10.0	2.00	3.50
B	TMWW-t	MT20	4.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TS-t	MT20	3.0	6.0		
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	5.0	6.0	2.50	2.75
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	8.0	2.25	4.00
I	BMWW-t	MT20	4.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMW+w	MT20	2.0	4.0		

NOTES (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
G	1337	0	1337	0	0	3-8	3-8	
A	1337	0	1337	0	0	MECHANICAL		2x4 L

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT A. MINIMUM BEARING LENGTH AT JOINT A = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	945	620 / 0	0 / 0	0 / 0	0 / 0	325 / 0	0 / 0
A	945	620 / 0	0 / 0	0 / 0	0 / 0	325 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.74 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, C-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- M	-3122 / 0	-91.8	-91.8	0.28 (1)	3.74	K- B	0 / 116	0.04 (4)
M- B	-2990 / 0	-91.8	-91.8	0.39 (1)	3.75	B- I	-921 / 0	0.67 (1)
B- C	-2086 / 0	-91.8	-91.8	0.35 (1)	4.39	I- C	0 / 446	0.10 (1)
C- D	-1103 / 0	-91.8	-91.8	0.38 (1)	5.55	C- H	-1144 / 0	0.41 (1)
D- E	-1103 / 0	-91.8	-91.8	0.38 (1)	5.55	H- E	-622 / 0	0.43 (1)
E- F	-1120 / 0	-91.8	-91.8	0.42 (1)	5.45	H- F	0 / 1756	0.40 (1)
G- F	-1289 / 0	0.0	0.0	0.43 (1)	5.68	L- M	0 / 99	0.00 (1)
A- L	0 / 2840	-18.5	-18.5	0.48 (1)	10.00			
L- K	0 / 2840	-18.5	-18.5	0.53 (1)	10.00			
K- J	0 / 2840	-18.5	-18.5	0.53 (1)	10.00			
J- I	0 / 2840	-18.5	-18.5	0.53 (1)	10.00			
I- H	0 / 1980	-18.5	-18.5	0.40 (1)	10.00			
H- G	0 / 0	-18.5	-18.5	0.16 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.27")

CSI: TC=0.43/1.00 (F-G:1) , BC=0.53/1.00 (I-K:1) , WB=0.67/1.00 (B-I:1) , SSI=0.25/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (H) (INPUT = 0.90)
JSI METAL= 0.93 (J) (INPUT = 1.00)

Structural component only
DWG# T-2136516

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420804	T20G	4	1	GOLDCOURT DEVELOPMENTS	

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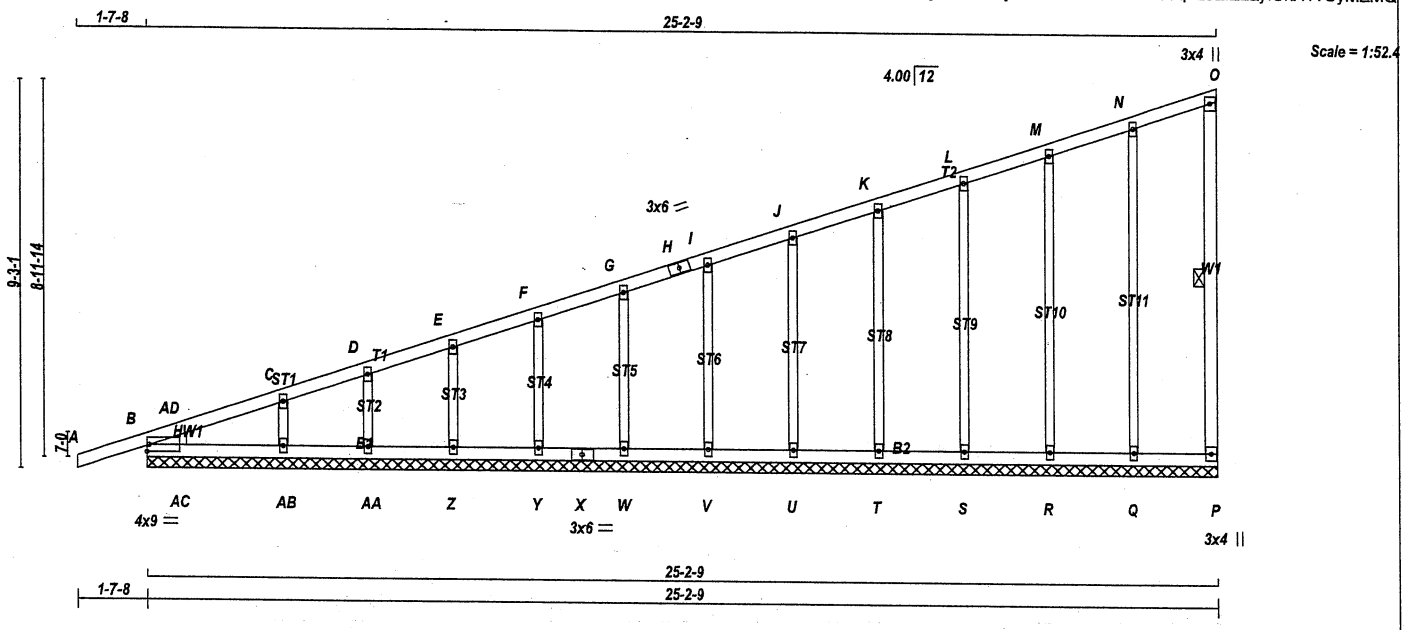

TOTAL WEIGHT = 4 X 111 = 445 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - H	2x4	DRY	No.2	SPF
H - O	2x4	DRY	No.2	SPF
P - O	2x4	DRY	No.2	SPF
B - X	2x4	DRY	No.2	SPF
X - P	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMBH1-l	MT20	4.0	9.0	Edge
C, D, E, F, G, I, J, K, L, M, N					
C	TMW+w	MT20	2.0	4.0	
H	TS-t	MT20	3.0	6.0	
O	TMV+p	MT20	3.0	4.0	
P	BMV1+p	MT20	3.0	4.0	
Q, R, S, T, U, V, W, Y, Z, AA, AB					
Q	BMW1+w	MT20	2.0	4.0	
X	BS-t	MT20	3.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

HEEL
WEDGE
2x4 L
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF O-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 12	-91.8	-91.8	0.18 (1)	10.00
B-AD	-49 / 0	-91.8	-91.8	0.03 (4)	6.25
AD- C	-20 / 0	-91.8	-91.8	0.09 (1)	6.25
C- D	-28 / 0	-91.8	-91.8	0.09 (1)	6.25
D- E	-16 / 0	-91.8	-91.8	0.04 (1)	6.25
E- F	-14 / 0	-91.8	-91.8	0.04 (1)	6.25
F- G	-11 / 0	-91.8	-91.8	0.04 (1)	6.25
G- H	-9 / 0	-91.8	-91.8	0.04 (1)	10.00
H- I	-9 / 0	-91.8	-91.8	0.04 (1)	10.00
I- J	-7 / 0	-91.8	-91.8	0.04 (1)	10.00
J- K	-5 / 0	-91.8	-91.8	0.04 (1)	10.00
K- L	-4 / 0	-91.8	-91.8	0.04 (1)	10.00
L- M	-2 / 0	-91.8	-91.8	0.04 (1)	10.00
M- N	0 / 0	-91.8	-91.8	0.05 (1)	10.00
N- O	-5 / 0	-91.8	-91.8	0.05 (1)	10.00
P- O	-77 / 0	0.0	0.0	0.03 (1)	6.25
B-AC	0 / 29	-18.5	-18.5	0.07 (1)	10.00
AC-AB	0 / 29	-18.5	-18.5	0.07 (1)	10.00
AB-AA	0 / 20	-18.5	-18.5	0.05 (1)	10.00
AA- Z	0 / 16	-18.5	-18.5	0.02 (4)	10.00
Z- Y	0 / 13	-18.5	-18.5	0.02 (4)	10.00
Y- X	0 / 10	-18.5	-18.5	0.02 (4)	10.00
X- W	0 / 10	-18.5	-18.5	0.02 (4)	10.00
W- V	0 / 8	-18.5	-18.5	0.01 (4)	10.00
V- U	0 / 7	-18.5	-18.5	0.01 (4)	10.00
U- T	0 / 5	-18.5	-18.5	0.01 (4)	10.00
T- S	0 / 4	-18.5	-18.5	0.01 (4)	10.00
S- R	0 / 2	-18.5	-18.5	0.01 (4)	10.00
R- Q	0 / 1	-18.5	-18.5	0.02 (4)	10.00
Q- P	0 / 0	-18.5	-18.5	0.02 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- N	-204 / 0	0.28 (1)
R- M	-181 / 0	0.20 (1)
S- L	-183 / 0	0.15 (1)
T- K	-183 / 0	0.12 (1)
U- J	-183 / 0	0.09 (1)
V- I	-183 / 0	0.07 (1)
W- G	-183 / 0	0.05 (1)
Y- F	-181 / 0	0.04 (1)
Z- E	-189 / 0	0.03 (1)
AA- D	-156 / 0	0.02 (1)
AB- C	-257 / 0	0.04 (1)
AC-AD	-78 / 4	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.18/1.00 (A-B:1) , BC=0.07/1.00 (B-AC:1) , WB=0.28/1.00 (N-Q:1) , SSI=0.12/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.51 (B) (INPUT = 0.90)
JSI METAL= 0.08 (C) (INPUT = 1.00)

Structural component only
DWG# T-2136517

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420804	T21	2	2	GOLDCOURT DEVELOPMENTS	

Tamarack Roof Truss, Burlington

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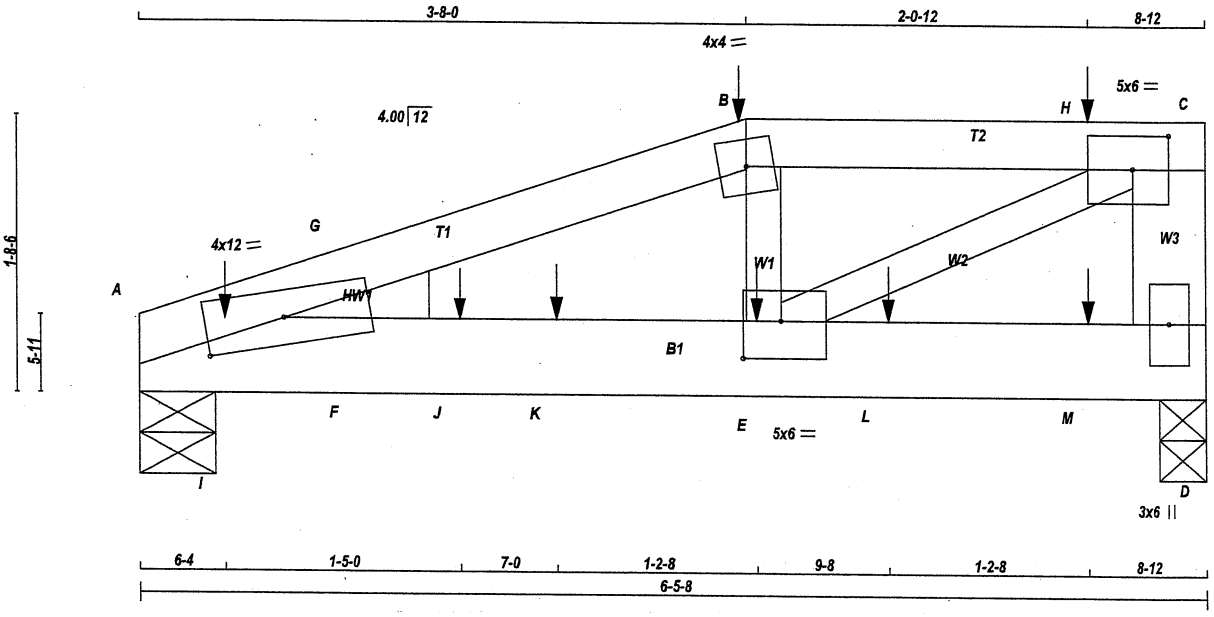


Scale = 1:13.5

TOTAL WEIGHT = 4 X 25 = 99 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
D - C	2x6	DRY	No.2	SPF
A - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- B	1	12	SIDE(61.0)
B- C	1	12	SIDE(61.0)
C- D	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
E- B	1	5	SIDE(50.7)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

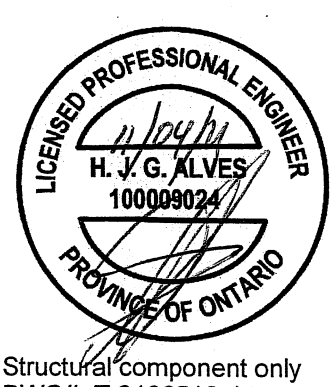
GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMBH1-m	MT20	4.0	12.0	2.00	5.75
B	TTW-m	MT20	4.0	4.0		
C	TMVW-t	MT20	5.0	6.0	2.50	2.75
D	BMV1+p	MT20	3.0	6.0		



Structural component only
DWG# T-2136518

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
D	2092	0	2092	0	0	3-8	3-8	
A	2697	0	2697	0	0	5-8	5-8	2x4 L

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	1477	982 / 0	0 / 0	0 / 0	0 / 0	495 / 0	0 / 0
A	1901	1280 / 0	0 / 0	0 / 0	0 / 0	621 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, A

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- G	-3639 / 0	-91.8	-91.8	0.18 (1)	4.75	E- B	0 / 628	0.08 (1)
G- B	-3482 / 0	-91.8	-91.8	0.19 (1)	4.83	E- C	0 / 3761	0.47 (1)
B- H	-3376 / 0	-91.8	-91.8	0.12 (1)	4.97	F- G	0 / 281	0.00 (1)
H- C	-3376 / 0	-91.8	-91.8	0.12 (1)	4.97			
D- C	-1828 / 0	0.0	0.0	0.07 (1)	7.81			
A- I	0 / 3326	-18.5	-18.5	0.44 (1)	10.00			
I- F	0 / 3326	-18.5	-18.5	0.44 (1)	10.00			
F- J	0 / 3326	-18.5	-18.5	0.55 (1)	10.00			
J- K	0 / 3326	-18.5	-18.5	0.55 (1)	10.00			
K- E	0 / 3326	-18.5	-18.5	0.55 (1)	10.00			
E- L	0 / 0	-18.5	-18.5	0.16 (1)	10.00			
L- M	0 / 0	-18.5	-18.5	0.16 (1)	10.00			
M- D	0 / 0	-18.5	-18.5	0.16 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
B	3-8-0	-107	-107	---	FRONT	VERT	TOTAL	---	C1
E	3-8-12	-31	-31	---	FRONT	VERT	TOTAL	---	C1
H	5-8-12	-43	-43	---	FRONT	VERT	TOTAL	---	C1
I	6-4	-799	-799	---	TOP	VERT	TOTAL	---	C1
J	1-11-4	-1	-1	---	FRONT	VERT	TOTAL	---	C1
K	2-6-4	-931	-931	---	BACK	VERT	TOTAL	---	C1
L	4-6-4	-931	-931	---	BACK	VERT	TOTAL	---	C1
M	5-8-12	-33	-33	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.19/1.00 (B-G:1) , BC=0.55/1.00 (E-F:1) , WB=0.47/1.00 (C-E:1) , SSI=0.38/1.00 (A-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (E) (INPUT = 0.90)
JSI METAL= 0.38 (C) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420804	T21	2	2	GOLDCOURT TRUSS DESC. DEVELOPMENTS	

Tamarack Roof Truss, Burlington

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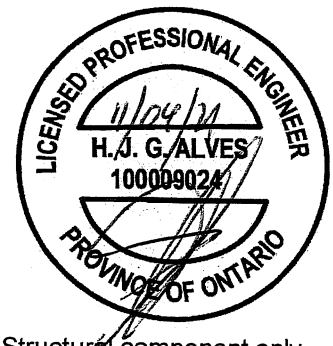
PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
E	BMWW-t	MT20	5.0	6.0	2.75	2.75

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.



Structural component only

DWG# T-2136518

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GOLDCOURT	DRWG NO.
420804	T22	2	1	TRUSS DESC.	DEVELOPMENTS	

Tamarack Roof Truss, Burlington

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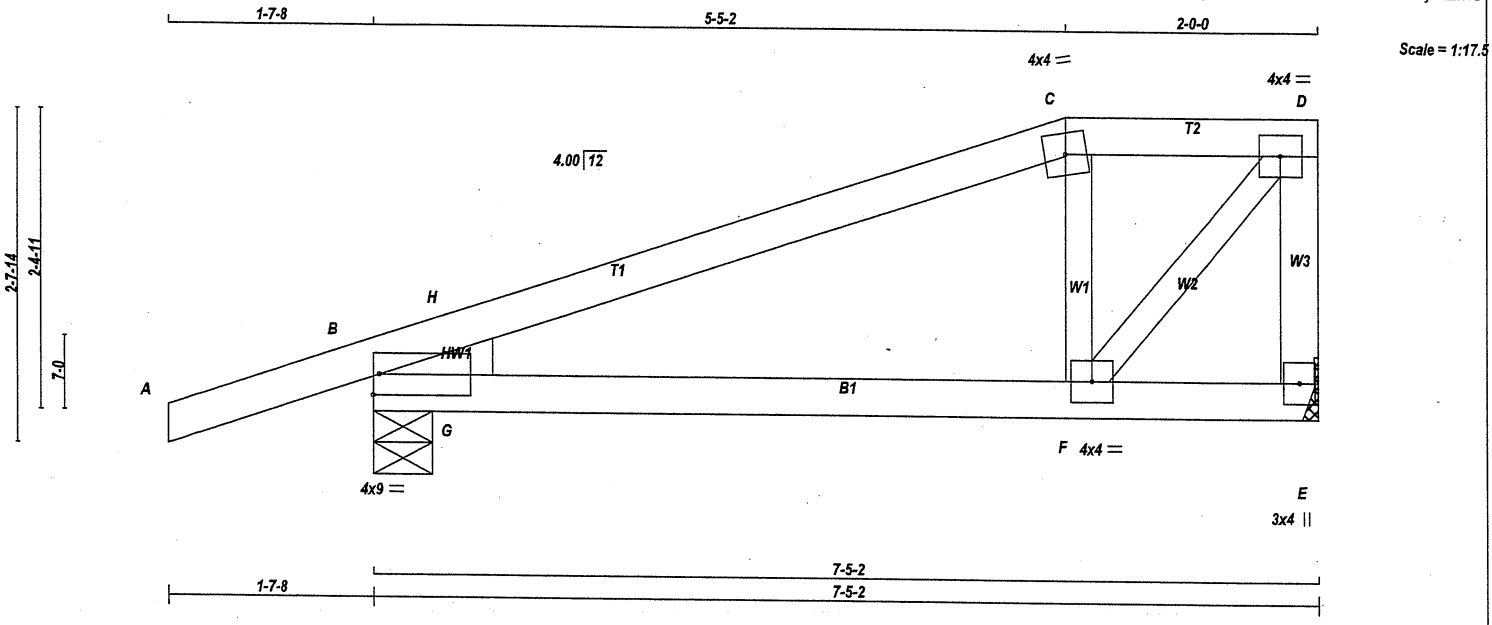

TOTAL WEIGHT = 2 X 26 = 52 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-I	MT20	4.0	9.0		Edge
C	TTW-m	MT20	4.0	4.0		
D	TMVW-t	MT20	4.0	4.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMWW-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
E	409	0	409	0	0	MECHANICAL		
B	563	0	563	0	0	5-8	5-8	2x4 L

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS: SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	290	190 / 0	0 / 0	0 / 0	0 / 0	100 / 0	0 / 0
B	395	276 / 0	0 / 0	0 / 0	0 / 0	120 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 12	-91.8	-91.8	0.18 (1)	10.00
B- H	-355 / 0	-91.8	-91.8	0.08 (1)	6.25
H- C	-359 / 0	-91.8	-91.8	0.29 (1)	6.25
C- D	-327 / 0	-91.8	-91.8	0.05 (1)	6.25
E- D	-453 / 0	0.0	0.0	0.06 (1)	7.81
B- G	0 / 336	-18.5	-18.5	0.27 (1)	10.00
G- F	0 / 336	-18.5	-18.5	0.27 (1)	10.00
F- E	0 / 0	-18.5	-18.5	0.16 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	-182 / 0	0.03 (1)
F- D	0 / 488	0.11 (1)
G- H	-234 / 12	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.25")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.25")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.29/1.00 (C-H:1) , BC=0.27/1.00 (B-G:1) , WB=0.11/1.00 (D-F:1) , SSI=0.18/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.67 (B) (INPUT = 0.90)
JSI METAL= 0.14 (F) (INPUT = 1.00)

Structural component only
DWG# T-2136519

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GOLDCOURT	DRWG NO.
420804	T23	2	1	TRUSS DESC.	DEVELOPMENTS	

Tamarack Roof Truss, Burlington

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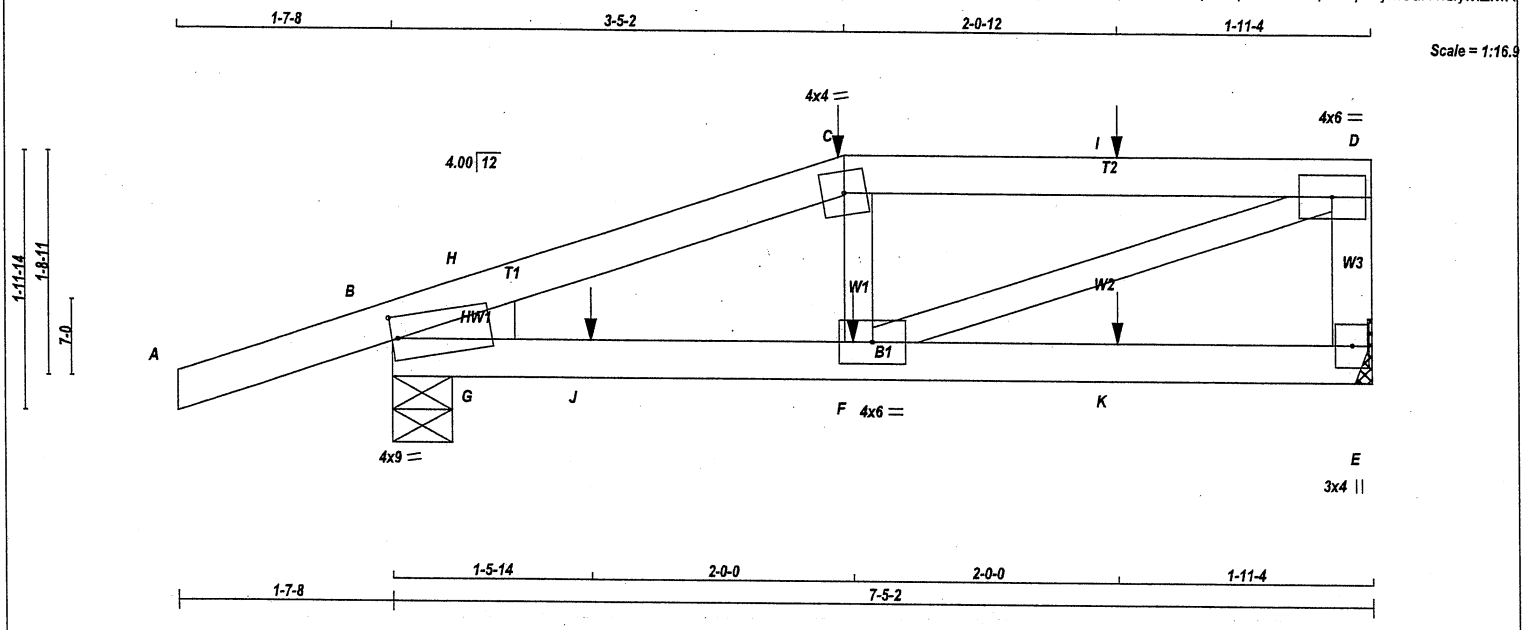


TOTAL WEIGHT = 2 X 25 = 51 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-m	MT20	4.0	9.0	2.00	0.50
C	TTW-m	MT20	4.0	4.0		
D	TMVW-t	MT20	4.0	6.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMWW-t	MT20	4.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
E	553	0	553	0	0	MECHANICAL		
B	700	0	700	0	0	5-8	5-8	2x4 L

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	391	257 / 0	0 / 0	0 / 0	0 / 0	134 / 0	0 / 0
B	492	339 / 0	0 / 0	0 / 0	0 / 0	153 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.20 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 12	-91.8	-91.8	0.19 (1)	10.00		F- C	-151 / 4	0.02 (1)
B- H	-962 / 0	-91.8	-91.8	0.10 (1)	6.20		F- D	0 / 853	0.21 (1)
H- C	-863 / 0	-91.8	-91.8	0.10 (1)	6.25		G- H	0 / 71	0.00 (1)
C- I	-801 / 0	-91.8	-91.8	0.36 (1)	6.20				
I- D	-801 / 0	-91.8	-91.8	0.36 (1)	6.20				
E- D	-502 / 0	0.0	0.0	0.06 (1)	7.81				
B- G	0 / 812	-18.5	-18.5	0.16 (1)	10.00				
G- J	0 / 812	-18.5	-18.5	0.18 (1)	10.00				
J- F	0 / 812	-18.5	-18.5	0.18 (1)	10.00				
F- K	0 / 0	-18.5	-18.5	0.10 (1)	10.00				
K- E	0 / 0	-18.5	-18.5	0.10 (1)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-5-2	-8	-8	---	FRONT	VERT	DEAD	---	C1
C	3-5-2	-39	-39	---	FRONT	VERT	TOTAL	---	C1
C	3-5-2	-31	-31	---	FRONT	VERT	SNOW	---	C1
F	3-5-14	-29	-29	---	FRONT	VERT	TOTAL	---	C1
I	5-5-14	-33	-33	---	FRONT	VERT	TOTAL	---	C1
J	1-5-14	-29	-29	---	FRONT	VERT	TOTAL	---	C1
K	5-5-14	-29	-29	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.25")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.25")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.36/1.00 (C-D:1) , BC=0.18/1.00 (F-G:1) , WB=0.21/1.00 (D-F:1) , SSI=0.18/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

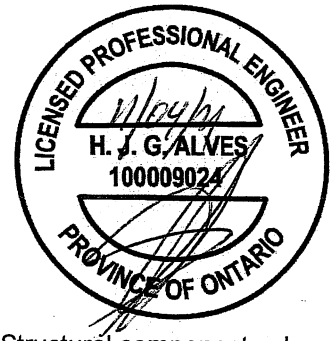
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.51 (F) (INPUT = 0.90)
JSI METAL= 0.19 (D) (INPUT = 1.00)



Structural component only
DWG# T-2136520

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420804	J1	12	1	GOLDCOURT	
				TRUSS DESC. DEVELOPMENTS	

Tamarack Roof Truss, Burlington

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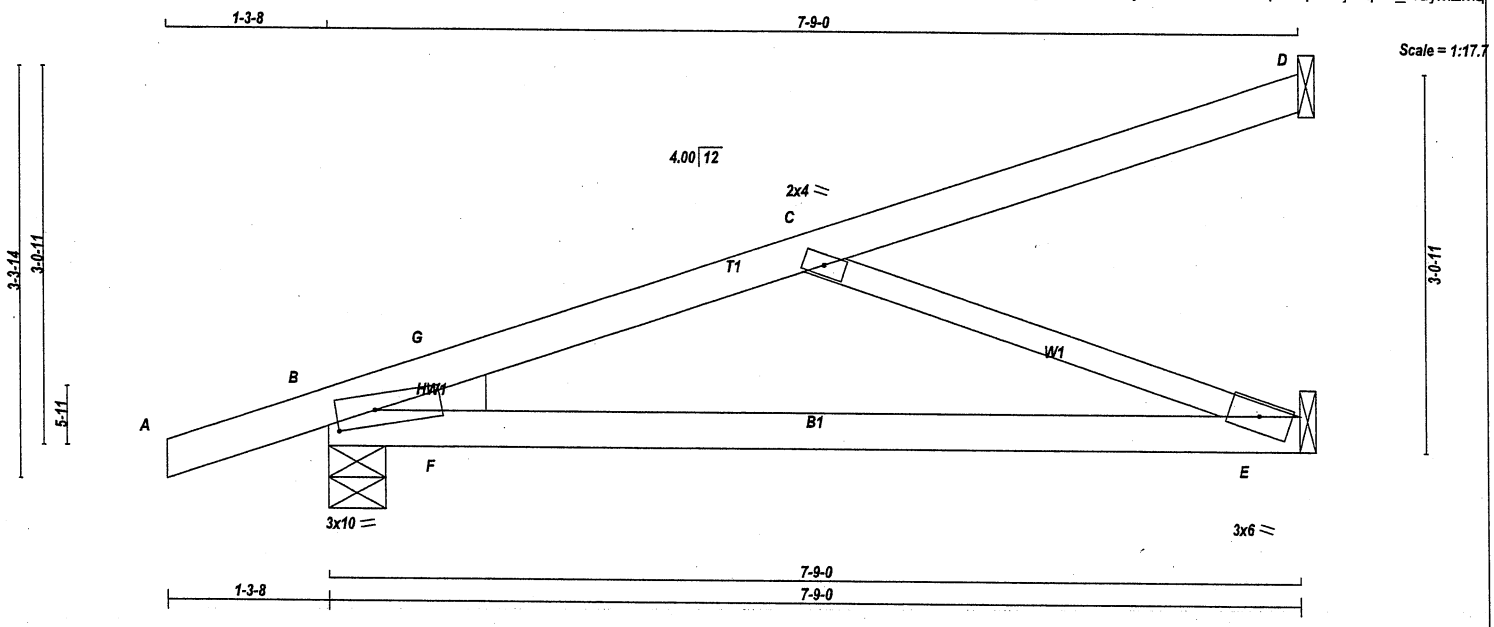

TOTAL WEIGHT = 12 X 24 = 290 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-m	MT20	3.0	10.0	1.50	3.75
C	TMW+w	MT20	2.0	4.0		
E	BMW1+w	MT20	3.0	6.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
D	135	0	135	0	0	1-8	1-8	
B	538	0	538	0	0	5-8	5-8	2x4 L
E	299	0	299	0	0	1-8	1-8	

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) D , E

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	92	79 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0
B	378	262 / 0	0 / 0	0 / 0	0 / 0	116 / 0	0 / 0
E	214	124 / 0	0 / 0	0 / 0	0 / 0	90 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO					FR-TO			
A- B	0 / 12	-91.8	-91.8	0.11 (1)	10.00	C- E	-626 / 0	0.20 (1)
B- G	-692 / 0	-91.8	-91.8	0.15 (4)	6.25	F- G	0 / 142	0.00 (1)
G- C	-583 / 0	-91.8	-91.8	0.17 (1)	6.25			
C- D	-13 / 0	-91.8	-91.8	0.17 (1)	6.25			
B- F	0 / 578	-18.5	-18.5	0.15 (4)	10.00			
F- E	0 / 578	-18.5	-18.5	0.22 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.25")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.25")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.17/1.00 (C-G:1) , BC=0.22/1.00 (E-F:4) , WB=0.20/1.00 (C-E:1) , SSI=0.16/1.00 (C-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.52 (C) (INPUT = 0.90)
JSI METAL= 0.31 (C) (INPUT = 1.00)

Structural component only
DWG# T-2136491

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420804	J2	12	1	GOLDCOURT	
				TRUSS DESC. DEVELOPMENTS	

Tamarack Roof Truss, Burlington

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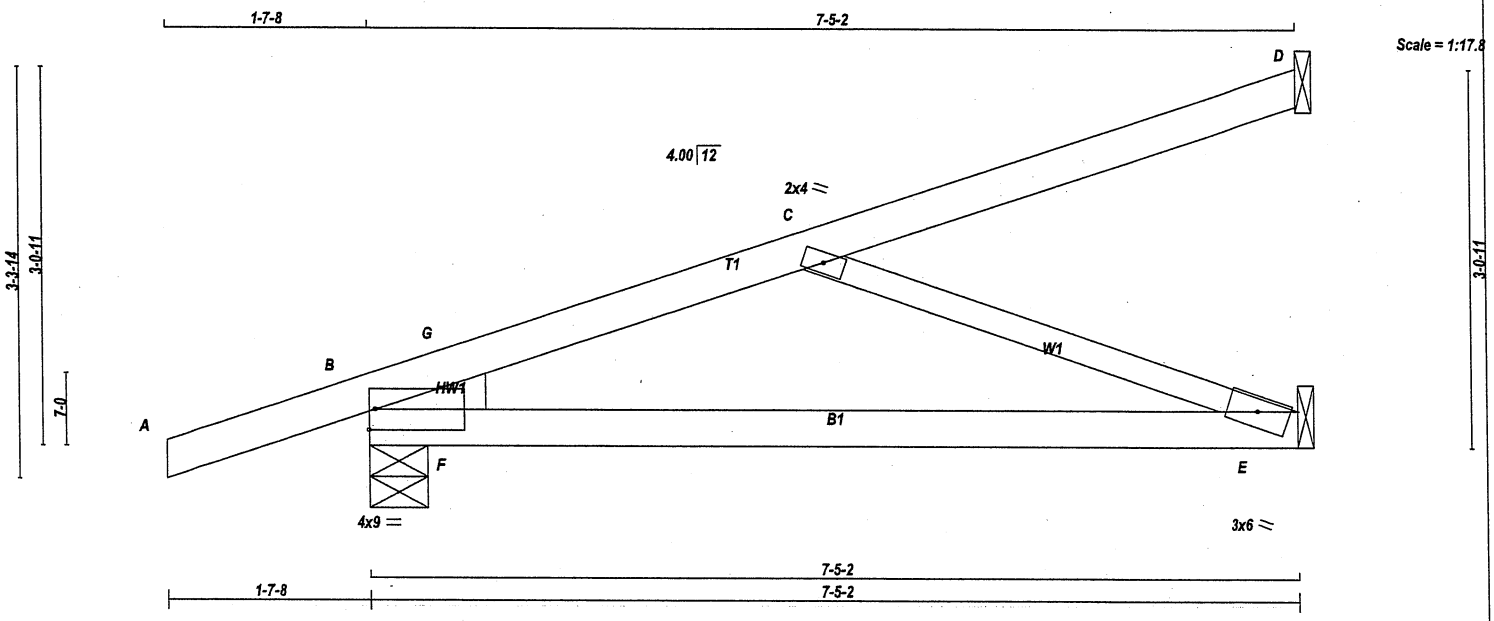

TOTAL WEIGHT = 12 X 24 = 286 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMBH1-I	MT20	4.0	9.0	Edge
C	TMW+w	MT20	2.0	4.0	
E	BMW1+w	MT20	3.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
D	141	0	141	0	0	1-8	1-8	
B	551	0	551	0	0	5-8	5-8	2x4 L
E	275	0	275	0	0	1-8	1-8	

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) D , E

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	96	82 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0
B	387	271 / 0	0 / 0	0 / 0	0 / 0	116 / 0	0 / 0
E	197	113 / 0	0 / 0	0 / 0	0 / 0	84 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 12	-91.8	-91.8	0.18 (1)	10.00	C- E	-576 / 0	0.19 (1)
B- G	-685 / 0	-91.8	-91.8	0.12 (4)	6.25	F- G	0 / 174	0.00 (1)
G- C	-542 / 0	-91.8	-91.8	0.14 (1)	6.25			
C- D	-11 / 0	-91.8	-91.8	0.14 (1)	6.25			
B- F	0 / 531	-18.5	-18.5	0.17 (4)	10.00			
F- E	0 / 531	-18.5	-18.5	0.20 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.24")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.24")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.18/1.00 (A-B:1) , BC=0.20/1.00 (E-F:4) , WB=0.19/1.00 (C-E:1) , SSI=0.16/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.63 (B) (INPUT = 0.90)
JSI METAL= 0.29 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
11/04/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2136492

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420804	J3	6	1	GOLDCOURT DEVELOPMENTS	

Tamarack Roof Truss, Burlington

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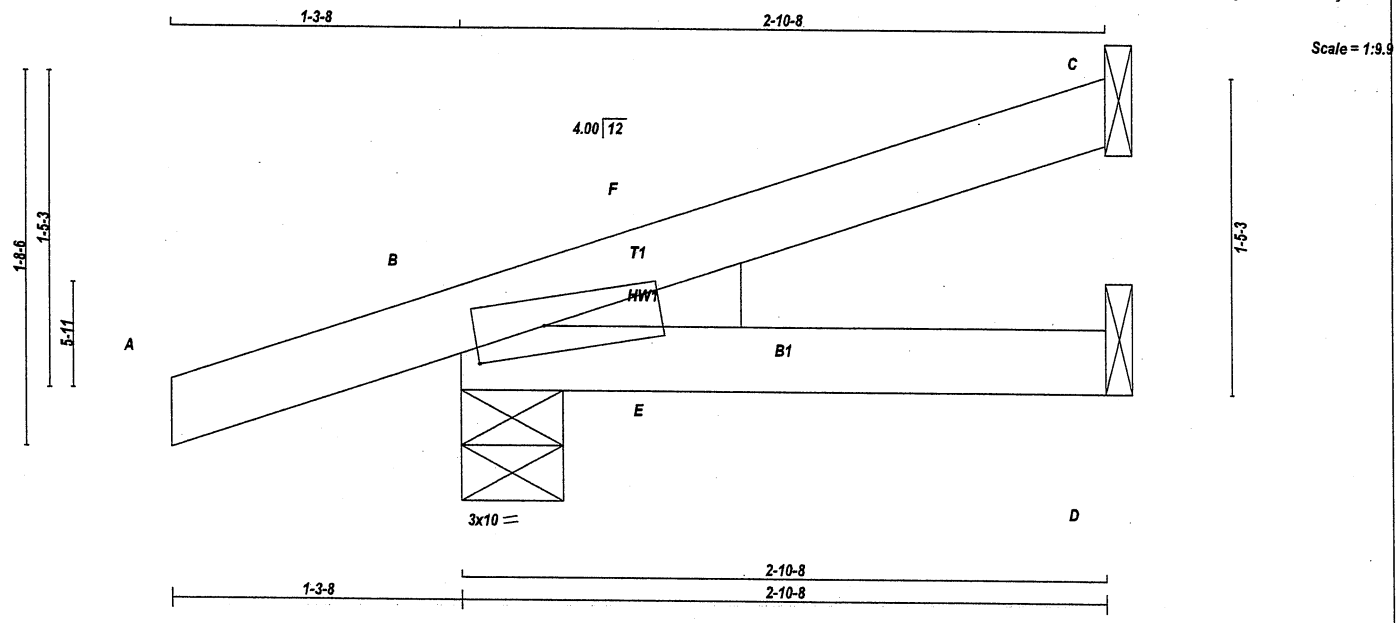

TOTAL WEIGHT = 6 X 9 = 56 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-m	MT20	3.0	10.0	1.50	3.75

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
C	107	0	107	0	0	1-8	1-8	
B	281	0	281	0	0	5-8	5-8	2x4 L
D	52	0	52	0	0	1-8	1-8	

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	74	56 / 0	0 / 0	0 / 0	0 / 0	18 / 0	0 / 0
B	197	142 / 0	0 / 0	0 / 0	0 / 0	55 / 0	0 / 0
D	38	17 / 0	0 / 0	0 / 0	0 / 0	21 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 12	-91.8	-91.8	0.12 (5)	10.00	E- F	-76 / 3	0.00 (1)
B- F	-17 / 0	-91.8	-91.8	0.05 (1)	6.25			
F- C	0 / 6	-91.8	-91.8	0.09 (1)	10.00			
B- E	0 / 0	-18.5	-18.5	0.09 (1)	10.00			
E- D	0 / 0	-18.5	-18.5	0.09 (1)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.12/1.00 (A-B:5) , BC=0.09/1.00 (D-E:1) , WB=0.00/1.00 (E-F:1) , SSI=0.10/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.08 (B) (INPUT = 0.90)
JSI METAL= 0.02 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2136493

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420804	J4	4	1	GOLDCOURT TRUSS DESC. DEVELOPMENTS	

Tamarack Roof Truss, Burlington

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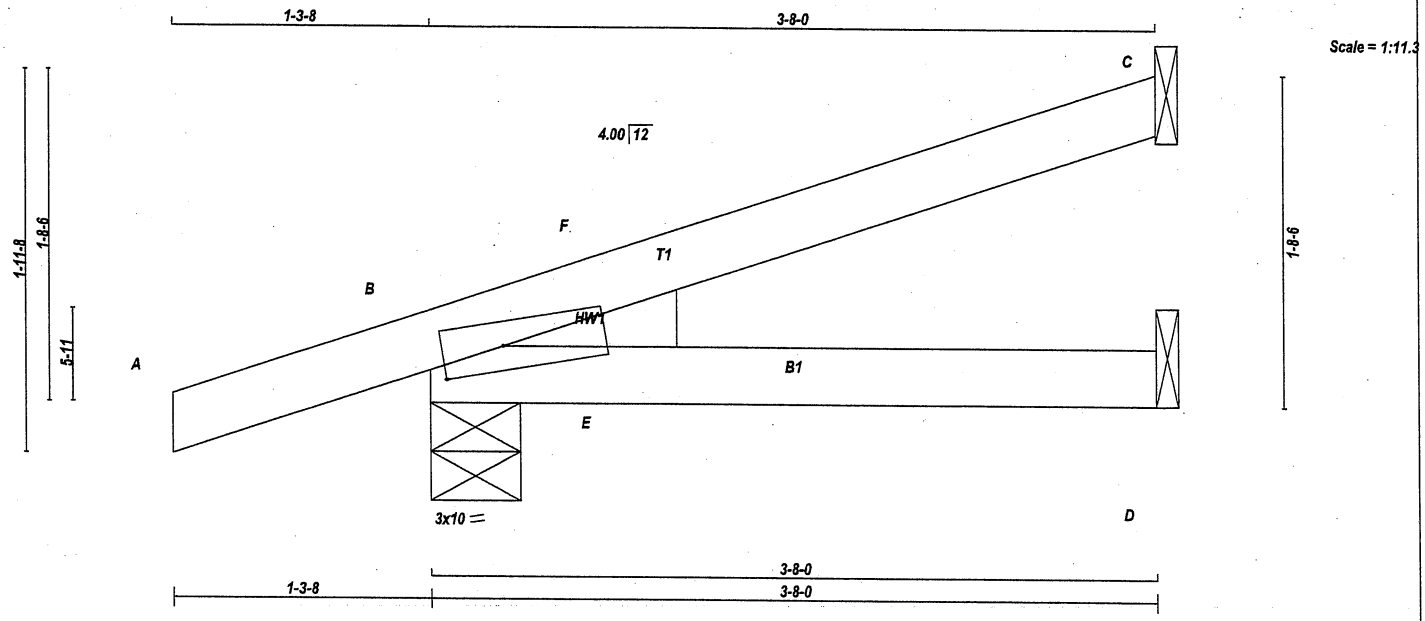

TOTAL WEIGHT = 4 X 11 = 45 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-m	MT20	3.0	10.0	1.50	3.75

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
C	140	0	140	0	0	1-8	1-8	
B	325	0	325	0	0	5-8	5-8	2x4 L
D	62	0	62	0	0	1-8	1-8	

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	97	75 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
B	227	162 / 0	0 / 0	0 / 0	0 / 0	65 / 0	0 / 0
D	46	19 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	CSI	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 12	-91.8	-91.8	0.13 (5)	10.00	E- F	-110 / 4	0.00 (1)	
B- F	-20 / 0	-91.8	-91.8	0.06 (1)	6.25				
F- C	0 / 5	-91.8	-91.8	0.15 (1)	10.00				
B- E	0 / 0	-18.5	-18.5	0.14 (1)	10.00				
E- D	0 / 0	-18.5	-18.5	0.14 (1)	10.00				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.15/1.00 (C-F:1) , BC=0.14/1.00 (B-E:1) , WB=0.00/1.00 (E-F:1) , SSI=0.11/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.09 (B) (INPUT = 0.90)
JSI METAL= 0.02 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2136494

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420804	J4G	2	1	GOLDCOURT DEVELOPMENTS	

Tamarack Roof Truss, Burlington

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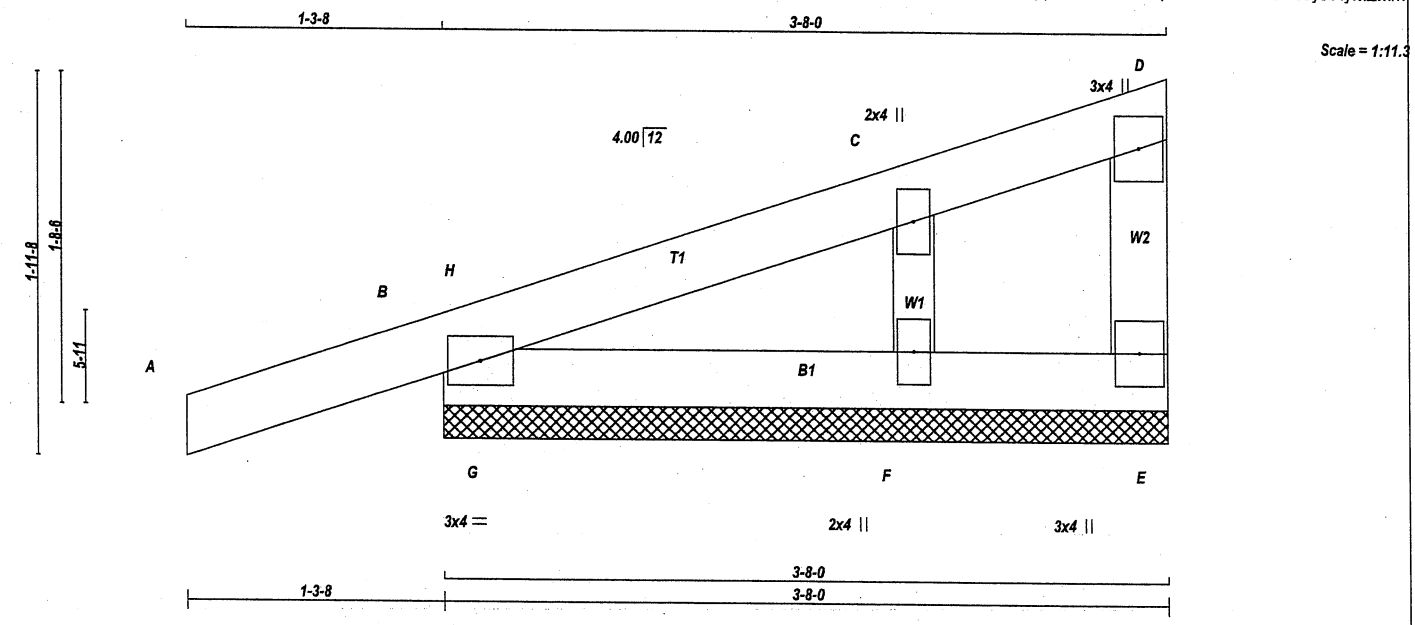

TOTAL WEIGHT = 2 X 12 = 24 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TMW+w	MT20	2.0	4.0		
D	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMW1+w	MT20	2.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 12	-91.8	-91.8	0.13 (5)	10.00		F- C	-198 / 0	0.03 (1)
B- H	-14 / 0	-91.8	-91.8	0.01 (4)	6.25		G- H	-86 / 4	0.00 (1)
H- C	-4 / 0	-91.8	-91.8	0.05 (1)	10.00				
C- D	-9 / 0	-91.8	-91.8	0.05 (1)	10.00				
E- D	-33 / 0	0.0	0.0	0.00 (1)	7.81				
B- G	0 / 10	-18.5	-18.5	0.04 (1)	10.00				
G- F	0 / 10	-18.5	-18.5	0.04 (1)	10.00				
F- E	0 / 0	-18.5	-18.5	0.02 (1)	10.00				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.13/1.00 (A-B:5) , BC=0.04/1.00 (F-G:1) , WB=0.03/1.00 (C-F:1) , SSI=0.11/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (B) (INPUT = 0.90)
JSI METAL= 0.06 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
11/04/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2136495

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420804	J5	6	1	GOLDCOURT DEVELOPMENTS	

Tamarack Roof Truss, Burlington

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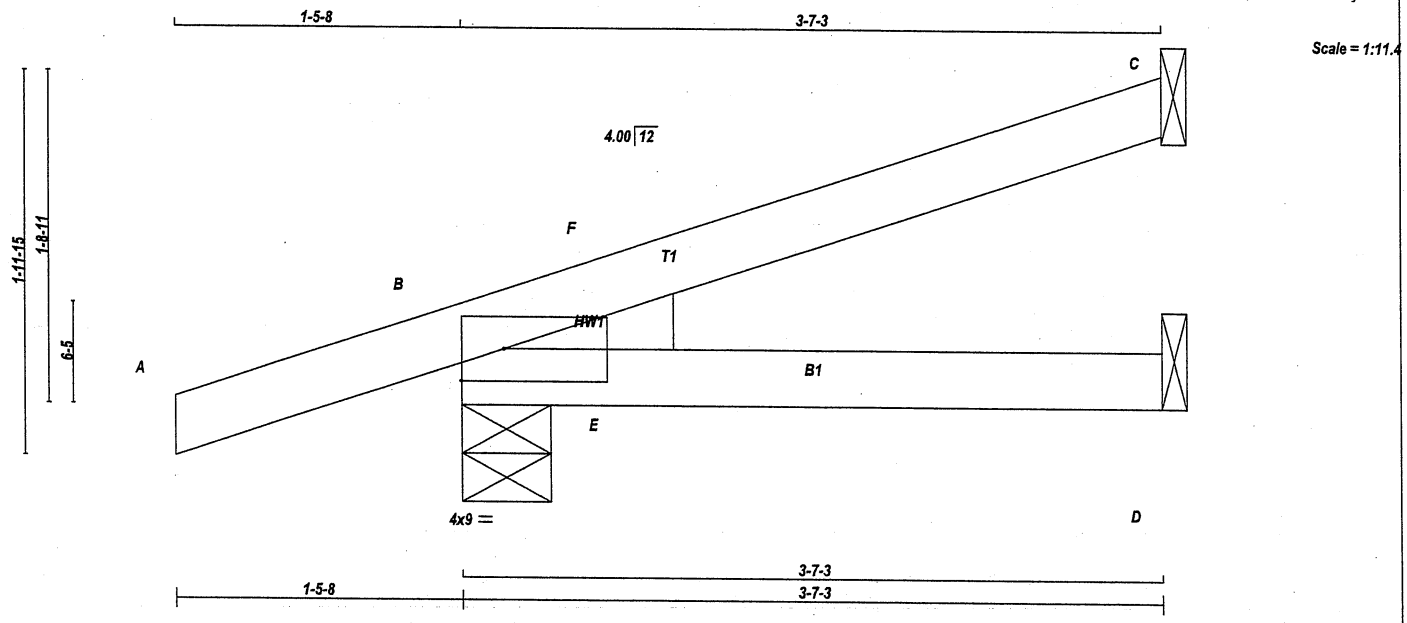

TOTAL WEIGHT = 6 X 11 = 68 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-I	MT20	4.0	9.0		Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
C	139	0	139	0	0	1-8	1-8	
B	336	0	336	0	0	5-8	5-8	2x4 L
D	59	0	59	0	0	1-8	1-8	

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN: COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	96	74 / 0	0 / 0	0 / 0	0 / 0	22 / 0	0 / 0
B	235	169 / 0	0 / 0	0 / 0	0 / 0	66 / 0	0 / 0
D	44	18 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, B

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 12	-91.8	-91.8	0.16 (5)	10.00	E- F	-115 / 4	0.00 (1)
B- F	-20 / 0	-91.8	-91.8	0.05 (4)	6.25			
F- C	0 / 4	-91.8	-91.8	0.14 (1)	10.00			
B- E	0 / 0	-18.5	-18.5	0.13 (1)	10.00			
E- D	0 / 0	-18.5	-18.5	0.13 (1)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.16/1.00 (A-B:5) , BC=0.13/1.00 (B-E:1) , WB=0.00/1.00 (E-F:1) , SSI=0.12/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (B) (INPUT = 0.90)
JSI METAL= 0.02 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2136496

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420804	C1	4	1	GOLDCOURT DEVELOPMENTS	

Tamarack Roof Truss, Burlington

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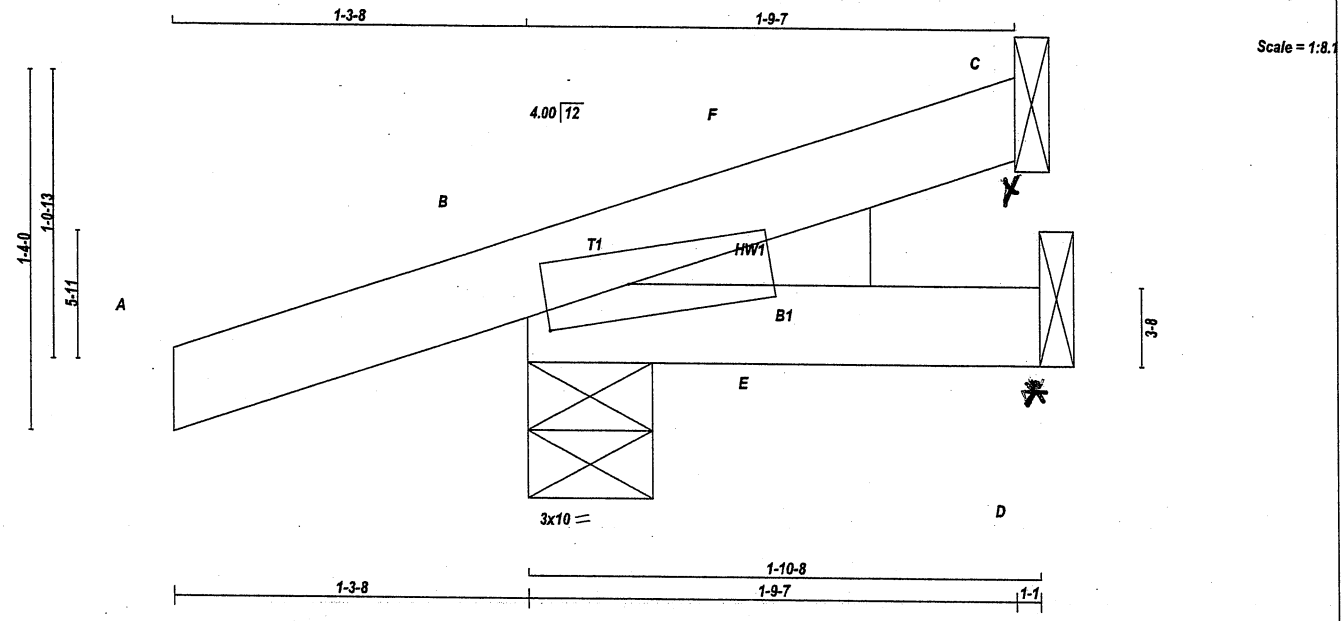

TOTAL WEIGHT = 4 X 7 = 28 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-m	MT20	3.0	10.0	1.50	3.75

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG	HEEL
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	WEDGE
C	50	0	50	0	0	1-8	1-8	
B	267	0	267	0	0	5-8	5-8	2x4 L
D	4	0	12	0	-21	1-8	1-8	

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	35	25 / -6	0 / 0	0 / 0	0 / 0	10 / 0	0 / 0
B	186	140 / 0	0 / 0	0 / 0	0 / 0	46 / 0	0 / 0
D	4	0 / -18	0 / 0	0 / 0	0 / 0	8 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, B

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS						WEBS		
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 12	-91.8	-91.8	0.11 (1)	10.00	E- F	0 / 68	0.00 (1)
B- F	-45 / 0	-91.8	-91.8	0.11 (1)	6.25			
F- C	-2 / 4	-91.8	-91.8	0.02 (1)	10.00			
B- E	0 / 0	-18.5	-18.5	0.04 (5)	10.00			
E- D	0 / 0	-18.5	-18.5	0.04 (5)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.11/1.00 (A-B:1) , BC=0.04/1.00 (B-E:5) , WB=0.00/1.00 (E-F:1) , SSI=0.10/1.00 (B-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.06 (B) (INPUT = 0.90)
JSI METAL= 0.01 (B) (INPUT = 1.00)

Structural component only
DWG# T-2136488

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420804	C2	2	1	GOLDCOURT DEVELOPMENTS	

Tamarack Roof Truss, Burlington

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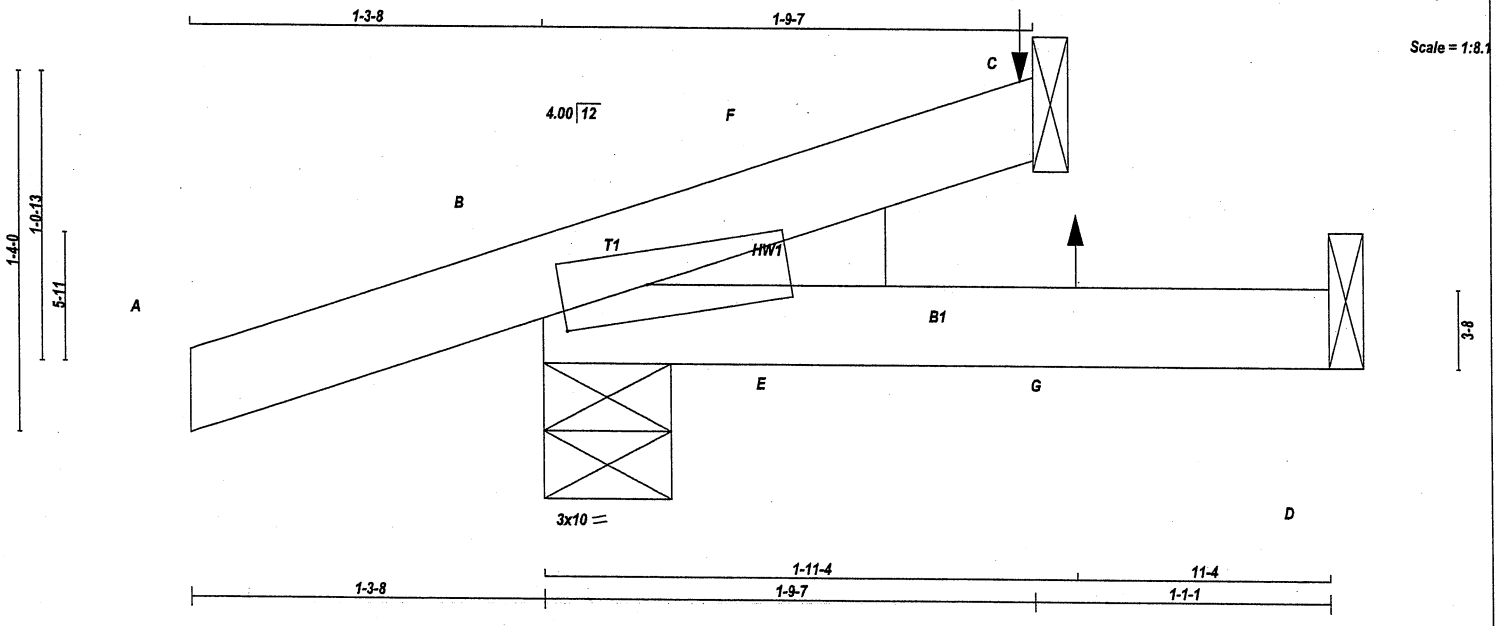

TOTAL WEIGHT = 2 X 8 = 16 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-m	MT20	3.0	10.0	1.50	3.75

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
C	64	0	64	0	-5	1-8	1-8	
B	269	0	269	0	0	5-8	5-8	2x4 L
D	12	0	19	0	0	1-8	1-8	

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	46	24 / -16	0 / 0	0 / 0	0 / 0	22 / 0	0 / 0
B	187	138 / 0	0 / 0	0 / 0	0 / 0	49 / 0	0 / 0
D	10	0 / -10	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, B

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (9)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 12	-91.8	-91.8	0.13 (5)	10.00	E- F	0 / 51	0.00 (1)
B- F	-46 / 0	-91.8	-91.8	0.13 (5)	6.25			
F- C	-4 / 5	-91.8	-91.8	0.03 (4)	10.00			
B- E	0 / 0	-18.5	-18.5	0.03 (5)	10.00			
E- G	0 / 0	-18.5	-18.5	0.03 (5)	10.00			
G- D	0 / 0	-18.5	-18.5	0.03 (5)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	1-9-7	-9	-9	23	BACK	VERT	TOTAL	—	C1
G	1-11-4	5	1	18	BACK	VERT	TOTAL	—	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.13/1.00 (A-B:5) , BC=0.03/1.00 (D-E:5) , WB=0.00/1.00 (E-F:1) , SSI=0.11/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.07 (B) (INPUT = 0.90)
JSI METAL= 0.01 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
11/04/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2136489

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420804	C3	2	1	GOLDCOURT TRUSS DESC. DEVELOPMENTS	

Tamarack Roof Truss, Burlington

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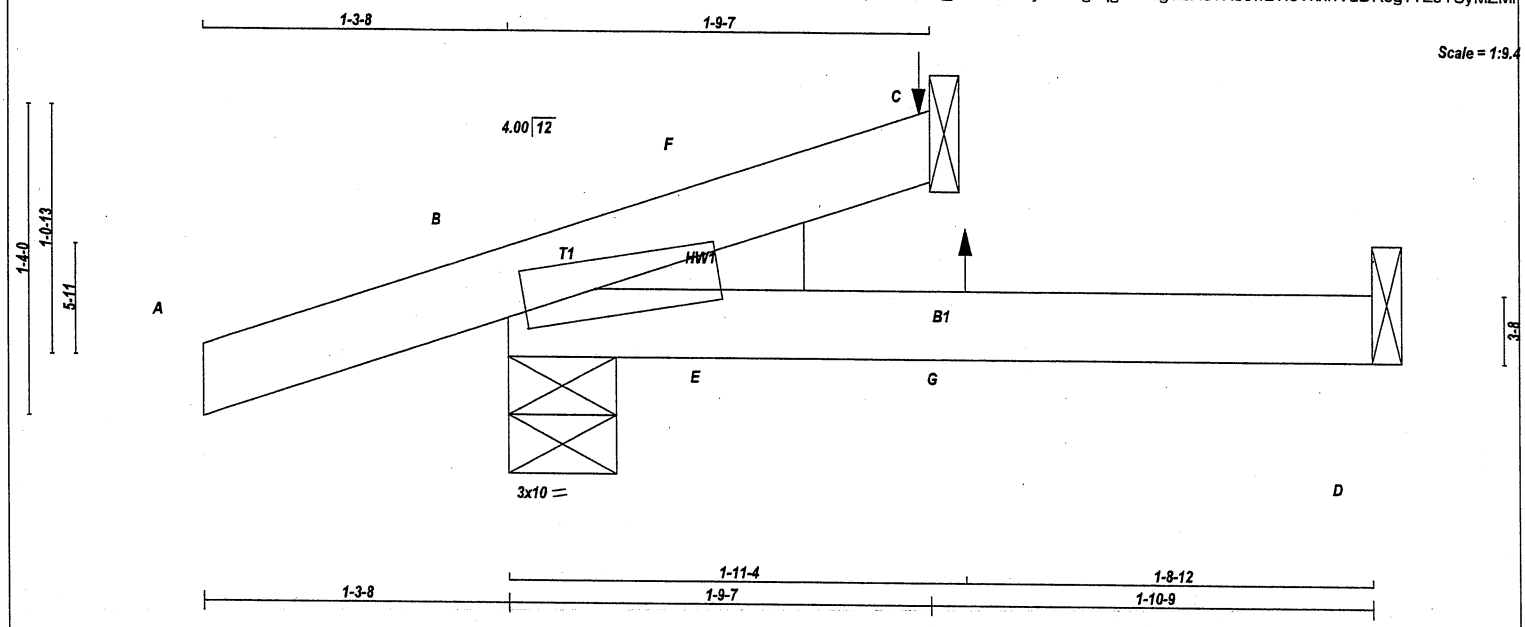


TOTAL WEIGHT = 2 X 9 = 18 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-m	MT20	3.0	10.0	1.50	3.75

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
C	70	0	70	0	-3	1-8	1-8	
B	270	0	270	0	0	5-8	5-8	2x4 L
D	19	0	25	0	0	1-8	1-8	

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
C	51	23 / -19	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0
B	188	138 / 0	0 / 0	0 / 0	0 / 0	50 / 0	0 / 0
D	16	0 / -6	0 / 0	0 / 0	0 / 0	18 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, B

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (9)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 12	-91.8	-91.8	0.13 (5)	10.00	E- F	0 / 53	0.00 (1)
B- F	-48 / 0	-91.8	-91.8	0.13 (5)	6.25			
F- C	-4 / 8	-91.8	-91.8	0.05 (4)	10.00			
B- E	0 / 0	-18.5	-18.5	0.03 (5)	10.00			
E- G	0 / 0	-18.5	-18.5	0.03 (4)	10.00			
G- D	0 / 0	-18.5	-18.5	0.03 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	1-9-7	-9	-9	23	BACK	VERT	TOTAL	---	C1
G	1-11-4	5	1	18	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.13/1.00 (A-B:5) , BC=0.03/1.00 (D-E:4) , WB=0.00/1.00 (E-F:1) , SSI=0.11/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

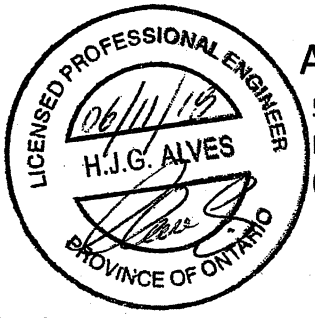
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.07 (B) (INPUT = 0.90)
JSI METAL= 0.01 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2136490



Alves Engineering Services Inc.
5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components

2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.

3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.

4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.

5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.

2- Lumber is to be the sizes and grade specified on the truss drawing.

3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.

4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings

5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.

6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)

7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.

8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1800218 Feb 09, 2018

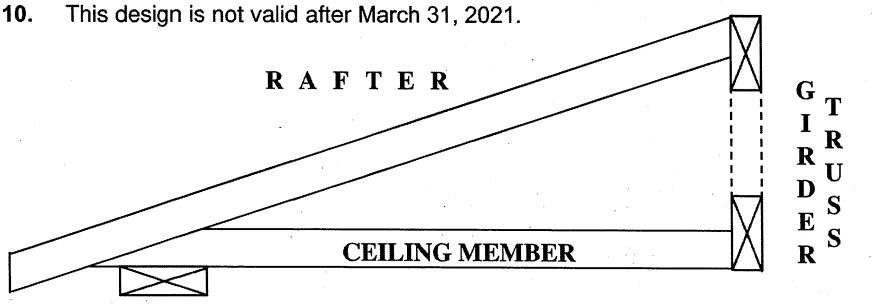
BEARING ANCHORAGE BY TOE-NAILS FOR LATERAL CAPACITY

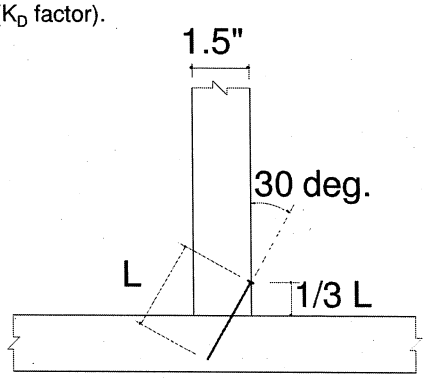
B97791H1

NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL LATERAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	132	147
	3.25	0.144	132	147
	3.50	0.160	159	177
COMMON SPIRAL	3.00	0.122	97	108
	3.25	0.122	97	108
	3.50	0.152	145	162

NOTES:

1. Rafter and ceiling members may be anchored to top and bottom chords of girder truss by toe-nailing rafter and ceiling members to girder chords provided the reaction does not exceed the lateral capacities in the table. Hangers (specified by others) are required for reactions higher than the maximum toe-nail capacity. Reactions are based on factored loads.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.4.1.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in tables below.
5. Nail values in table are based on the following relative lumber densities: G = 0.42 (SPF), G = 0.49 (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See next page for nailing on bearing plate).
7. For loads due to **wind** the nail lateral capacity in this table may be multiplied by 1.15 (K_D factor).
8. Lumber must be dry (< 19% moisture content) at the time of nail installation.
9. Nail values in this table comply with CSA O86-14, section 12.9.4
10. This design is not valid after March 31, 2021.





TOE-NAIL INSTALLATION

Nail type	Common wire	Common spiral	Common wire	Common spiral
Nail dia. (in)	0.160	0.152	0.144	0.122
	(3.5" nail)		(3" and 3.25" nail)	
LUMBER SIZE	MAXIMUM NUMBER OF TOE-NAILS			
2X4 SPF	2	2	3	3
2X4 D. Fir	2	2	2	2
2X6 SPF	4	4	4	5
2X6 D. Fir	3	3	3	4

MiTek®
MiTek Canada Inc
100 Industrial Rd.
Bradford, Ontario L3Z 3G7

PEO
Certificate No. 10889485

LICENSED PROFESSIONAL ENGINEER
C. Cordogiannis
90553736
PROVINCE OF ONTARIO

December 2, 2019

BEARING ANCHORAGE BY TOE-NAILS FOR WIND LOADING

B97791H2

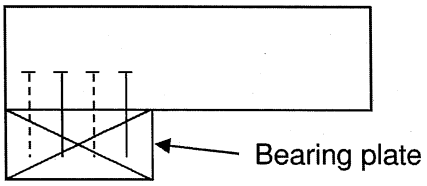
NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL WITHDRAWAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	30	42
	3.25	0.144	32	45
	3.50	0.160	38	52
COMMON SPIRAL	3.00	0.122	26	36
	3.25	0.122	28	40
	3.50	0.152	36	50

Note: If using truss with D. Fir lumber and S-P-F bearing plate, use values in table for S-P-F.

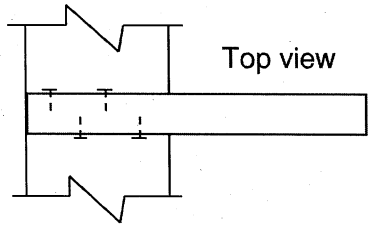
NOTES:

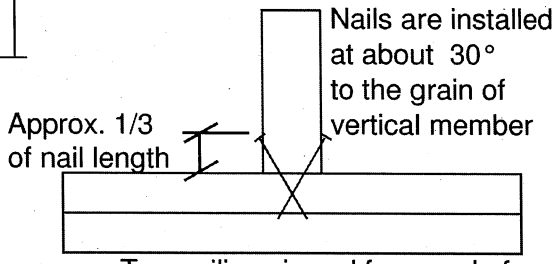
1. Truss chord, rafter, or ceiling members may be anchored to bearing plate by toe-nails, provided that the actual factored uplift force due to **wind** or **earthquake** load does not exceed the withdrawal capacities in the table. Hangers (specified by others) are required for uplift forces that are higher than the maximum toe-nail withdrawal capacity.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.5.2.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in table above.
5. Nail values in table are based on the following relative lumber densities: G = 0.42(SPF), G = 0.49(D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See drawing on detail B37579H1).
7. Lumber must be dry (< 19% moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-14, section 12.9.5
9. This design is not valid after March 31, 2021.

Toe-nailing on 2x6 Bearing Plate



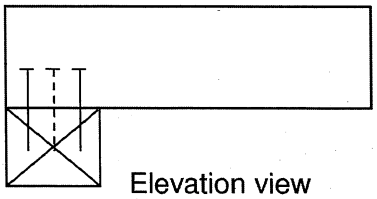
Elevation view



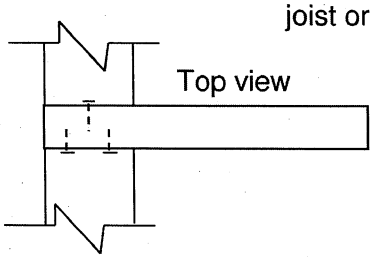


Toe-nailing viewed from end of joist or truss

Toe-nailing on 2x4 Bearing Plate



Elevation view



PEO
Certificate No. 10889485

MiTek®

MiTek Canada Inc
100 Industrial Rd.
Bradford, Ontario L3Z 3G7

December 2, 2019

TECHNICAL BULLETIN

HUS/LJS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

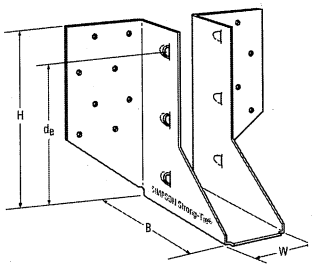
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

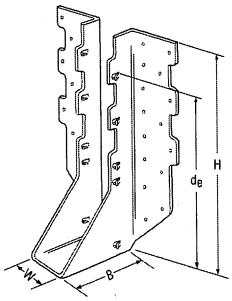
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

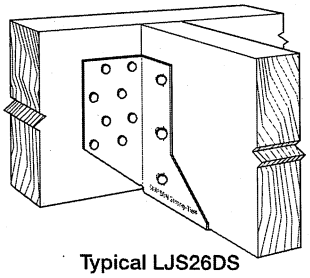
- See current catalogue for options



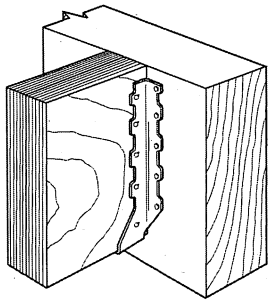
LJS26DS



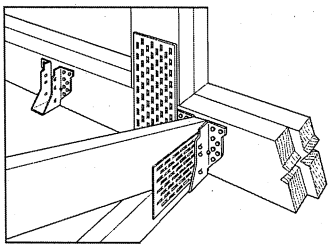
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS Installation



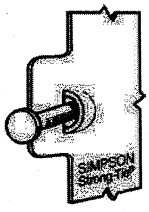
Typical HUS Installation



Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

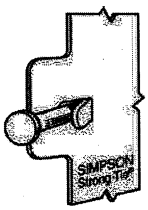
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
								lb.	lb.	lb.	lb.
LJS26DS	18	1 9/16	5	3½	4⅝	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1⅝	5⅜	3	3 15/16	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1⅝	7 3/32	3	6 3/32	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1⅝	9 3/32	3	7 31/32	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1 13/16	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

1. d_e is the distance from the seat of the hanger to the highest joist nail.

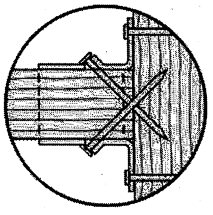


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

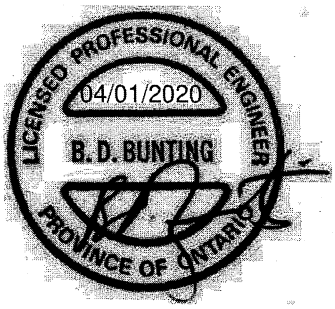
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

LUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

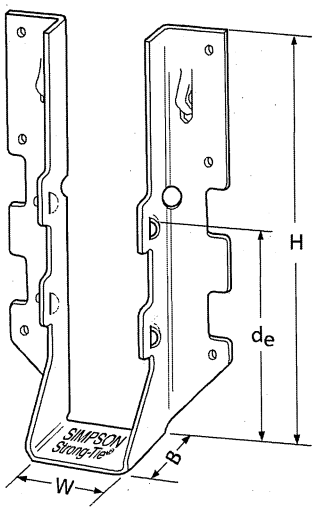
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

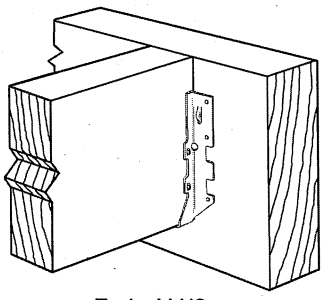
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified



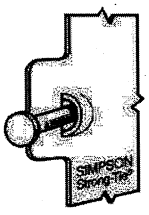
LUS28



Typical LUS Installation

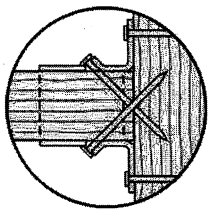
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LUS24	18	1 9/16	3 1/8	1 3/4	1 15/16	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/8	2	1 13/16	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1 9/16	4 3/4	1 3/4	3 5/8	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 7/8	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4 5/8	4 3/16	2	3 1/4	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1 9/16	6 5/8	1 3/4	3 3/4	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4 5/8	6 1/4	2	3 1/4	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1 9/16	7 13/16	1 3/4	3 7/8	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4 5/8	8 3/16	2	5 1/4	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.

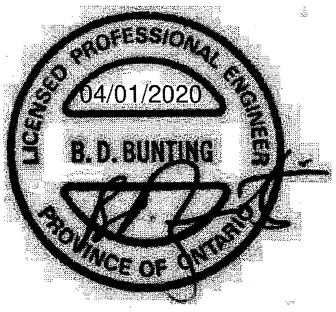


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Top View.



LIMIT STATES DESIGN

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T-SPECLUS20 3/20 exp. 6/22

(800) 999-5099
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TECHNICAL BULLETIN

HGUS – Double Shear Joist Hangers

SIMPSON Strong-Tie

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

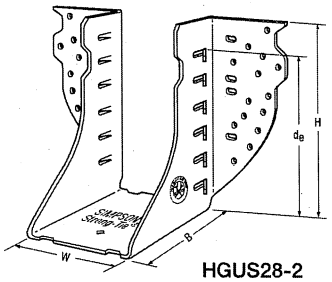
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

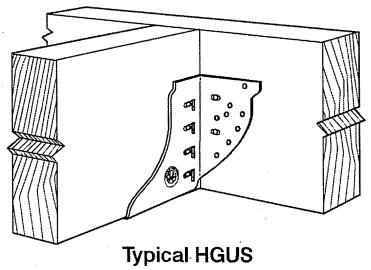
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

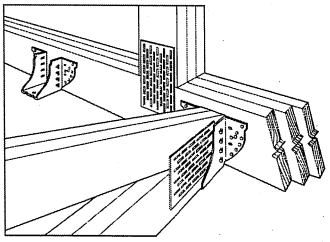
Options:

- See current catalogue for options



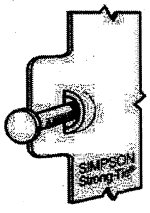
HGUS28-2


Typical HGUS Installation


Typical HGUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
HGUS26	12	1⅝	5⅜	5	4⅝₃₂	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3⅝₁₆	5⅞₁₆	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4¹⁵⁄₁₆	5½	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6⅜₁₆	5⅞₁₆	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1⅝	7⅛	5	6⅛	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3⅝₁₆	7⅜₁₆	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4¹⁵⁄₁₆	7¼	4	6⅜	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6⅜₁₆	7⅜₁₆	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210	12	1⅝	9⅛	5	7⅞	(46) 16d	(16) 16d	3535	11070	2510	8090
HGUS210-2	12	3⅝₁₆	9⅜₁₆	4	8⅛	(46) 16d	(16) 16d	6840	14015	4855	10270
HGUS210-3	12	4¹⁵⁄₁₆	9¼	4	8⅜	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6⅜₁₆	9⅜₁₆	4	8⅛	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6⅜₁₆	10⅝	4	10⅛	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6⅜₁₆	12⅝	4	11⅛	(66) 16d	(22) 16d	10130	16400	7195	11645

1. d_e is the distance from the seat of the hanger to the highest joist nail.



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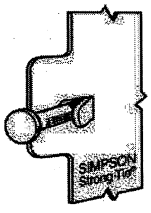
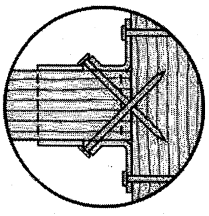


Double Shear Nailing Side View. Do not bend tab back.


Double Shear Nailing Top View.

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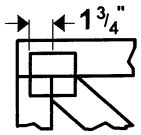
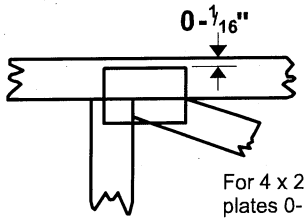
Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0- 1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

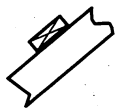
*** Plate location details available in MiTek 20/20 software or upon request.**

PLATE SIZE

4 x 4

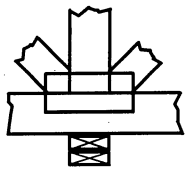
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING

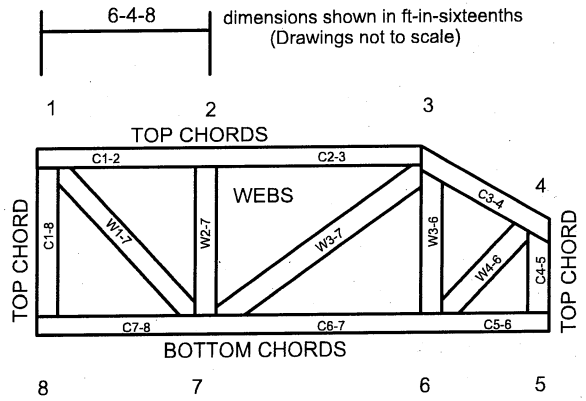


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur. Min size shown is for crushing only.

Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988
ER-3907, ESR-2362, ESR-1397, ESR-3282

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

MiTek

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.